



Better Utilization of Fly Ash and other SCMs in Lean Concrete and Durable Concrete Pavements and Structural Concrete Applications

A thesis by

Farzad Moghaddam

Supervisors: Prof Vute Sirivivatnanon and Dr Kirk Vessalas

A thesis submitted in fulfilment of the requirements for the degree of

Doctor of Philosophy

Faculty of Engineering and Information Technology

School of Civil and Environmental Engineering

University of Technology Sydney

May 2018

CERTIFICATE OF ORIGINAL AUTHORSHIP

I certify that the work in this thesis has not previously been submitted for a degree nor has it been submitted as part of requirements for a degree except fully acknowledged within the text.

I also certify that the thesis has been written by me. Any help that I have received in my research work and the preparation of the thesis itself has been acknowledged. In addition, I certify that all information sources and literature used are indicated in the thesis.

Farzad Moghaddam

Production Note:

Signature removed prior to publication.

Date: 08/05/2018

*Sincerely dedicated to
my lovely wife and son*

ACKNOWLEDGEMENTS

This thesis would not have been completed without the guidance, advice and support of a number of individuals whose contribution I would gratefully like to acknowledge. I would specially like to express my gratitude to my supervisors, *Professor Vute Sirivivatnanon* and *Dr Kirk Vessalas*. I would like to express my deepest gratitude to *Professor Vute Sirivivatnanon*, who has been my principal supervisor for this research, not only because of his invaluable guidance and expert advice throughout the research but also on account of his strong support, being a kind father during the last three and half years. The author would also very much like to record his appreciation to Dr Vessalas for his valuable advice and constant support from the commencement of this project to the end. I also greatly appreciate my retired co-supervisor, Dr. Rasiah Sri Ravindrarajah, for his mentorship and unfailing assistance and support throughout the course of this research. It should be mentioned that this research is supported by the Australian Government Research Training Program.

I would also like to convey my thanks to University of Technology Sydney (UTS) Civil Engineering Laboratories staff, specially the laboratories manager *Mr Rami Haddad*, who kindly helped me in project experimental stages.

Furthermore, I am obliged to many of my colleagues and friends who assisted me during my studies. Special thanks must go to my dear friends, Dr Amin Noushini, Mr Hamed Mahdavi, *Ms Marie Joshua Tapas* and *Mr Daniel John Pospischil* for their unfailing assistance and support in this research work.

My deepest gratitude is to my wife Ghazal for her unwavering understanding during all the years of this work, her selfless love and support throughout, and her encouragement and companionship during the preparation of this thesis. Her understanding of the demands required to complete this work has been inspirational. And finally to my son, Kian whose birth during the thesis preparation encouraged me to get it done as soon as I could.

Lastly, I would like to extend my love and gratitude to my dearest parents for their support and encouragement. I want to sincerely thank them from the bottom of my heart and acknowledge that without them none of this could have happened and I would not have been able to achieve most of the things I have in my life.

LIST OF PUBLICATIONS BASED ON THIS RESEARCH

- Moghaddam, F., Sri Ravindrarajah, R. & Sirivivatnanon, V. 2015, 'Properties of metakaolin concrete-a review', Paper presented to the International Conference on Sustainable Structural Concrete 2015, La Plata, Argentina.
- Moghaddam, F., Sri Ravindrarajah, R. 2016, 'Properties of fly ash concrete – a review ', Concrete Institute of Australia Journal. 42(4): p. 50-53
- Moghaddam, F., Sirivivatnanon, V., Vessalas, K. 2017, ' Effect of run-of-station fly ash and other SCMs on various properties of concrete ', Concrete Institute of Australia's Biennial National Conference, Adelaide, Australia.
- Moghaddam, F., Vessalas, K., Sirivivatnanon, V. 2017, ' Use of run-of-station fly ash and other SCMs in concrete pavement construction', Australian Society for Concrete Pavements 4th Concrete Pavements Conference, Kingscliff, Australia.
- Moghaddam, F., Sirivivatnanon, V., Vessalas, K. 2018, ' The effect of fly ash fineness on heat of hydration, microstructure, flow and compressive strength of blended cement pastes', Construction and Building Materials – under review
- Moghaddam, F., Vessalas, K., Sirivivatnanon, V. 2018, ' Investigation on the influence of supplementary cementitious materials on heat of hydration of blended cement pastes', ready for submission

TABLE OF CONTENTS

***ACKNOWLEDGEMENTS*..... iv**

***LIST OF PUBLICATIONS BASED ON THIS RESEARCH* v**

***TABLE OF CONTENTS*..... vi**

***LIST OF FIGURES*.....xii**

***LIST OF TABLES*xxi**

***ABBREVIATIONS*xxx**

***ABSTRACT*xxxii**

***1 INTRODUCTION* - 2 -**

1.1 Background to the project - 2 -

1.2 Research objective - 4 -

1.3 Specific objectives - 4 -

1.4 Organisation of dissertation - 5 -

2 LITERATURE REVIEW – INFLUENCE OF CHARACTERISTICS

AND LEVELS OF SCMS ON PROPERTIES OF MORTAR &

***CONCRETE*..... - 9 -**

2.1 Preface - 9 -

2.2 Physical characteristics and chemical composition of

SCMs..... - 9 -

2.3 Hydration and pozzolanic reaction - 14 -

2.4 Influence of type, fineness, and level of SCMs on

consistency and heat of hydration of mortar & concrete - 18 -

2.4.1	Consistency	- 18 -
2.4.2	Heat of hydration.....	- 20 -
2.5	Effect of type, fineness, and level of SCMs on hardened properties of mortar & concrete.....	- 29 -
2.5.1	Porosity and pore size distribution.....	- 29 -
2.5.2	Strength properties	- 37 -
2.5.3	Drying shrinkage.....	- 44 -
2.6	Influence of type, fineness, and level of SCMs on durability properties of mortar & concrete.....	- 46 -
2.6.1	Chloride resistance	- 46 -
2.6.2	Sulfate resistance.....	- 53 -
2.6.3	Alkali-silica mitigation.....	- 58 -
2.7	Summary	- 65 -
3	<i>RESEARCH METHODOLOGY.....</i>	- 70 -
3.1	Preface	- 70 -
3.1.1	Characterisation of cement, SCMs and aggregates.....	- 70 -
3.1.2	Determination of the effect of fineness and levels of SCMs on the heat of hydration.....	- 71 -
3.1.3	Effect of fly ash fineness on important properties of the blended cement paste	- 72 -
3.1.4	Determination of optimum binary binder systems in mortars	- 72 -

3.1.5	Effective level of SCMs in mitigating alkali-silica reactivity.....	- 73 -
3.1.6	Effectiveness of run-of-station fly ash in lean concrete.....	- 74 -
3.1.7	Effect of run-of-station fly ash and other SCMs on properties of high-performance concrete.....	- 74 -
3.2	Raw materials	- 75 -
3.3	Experimental procedures.....	- 76 -
3.3.1	Testing methods for characterisation of raw materials	- 76 -
3.3.2	Paste mix design and mixing procedure	- 78 -
3.3.3	Casting and curing paste specimens.....	- 79 -
3.3.4	Paste testing details	- 79 -
3.3.5	Mortar mixing procedure	- 83 -
3.3.6	Casting and curing and testing of mortar specimens	- 84 -
3.3.7	Concrete mix design process.....	- 85 -
3.3.8	Concrete mixing procedure	- 96 -
3.3.9	Casting, curing and testing of concrete specimens	- 96 -
3.3.10	Durability properties of concrete specimens.....	- 97 -
4	<i>CHARACTERISATION OF RAW MATERIALS</i>.....	- 99 -
4.1	Preface	- 99 -
4.1.1	Physical properties	- 99 -
4.1.2	Chemical composition.....	- 103 -
4.1.3	Identification of crystalline phase	- 106 -
4.1.4	Surface Topography	- 109 -

4.2	Aggregate	- 113 -
4.2.1	Grading.....	- 113 -
4.2.2	Water absorption and specific gravity.....	- 116 -
5	<i>EFFECT OF FINENESS AND LEVEL OF DIFFERENT FLY ASHES AND SCMs ON IMPORTANT PROPERTIES OF THE CEMENT PASTE</i>	- 118 -
5.1	Preface	- 118 -
5.2	Heat of hydration	- 118 -
5.2.1	Stage of Hydration: Overview	- 118 -
5.2.2	Effect of different fly ashes.....	- 120 -
5.2.3	Effect of slag	- 130 -
5.2.4	Effect of metakaolin.....	- 133 -
5.3	Calcium hydroxide consumption	- 136 -
5.4	Microstructure of hardened blended cement pastes .	- 139 -
5.5	Flow and strength properties	- 145 -
5.6	Summary	- 149 -
6	<i>EFFECT OF FINENESS AND LEVEL OF FLY ASH AND OTHER SCM ADDITIONS ON FRESH AND HARDENED PROPERTIES OF CEMENT MORTAR</i>	- 152 -
6.1	Preface	- 152 -
6.2	Effect of fineness and level of fly ash on important properties of mortar	- 152 -

6.2.1	Flow and wet density	- 152 -
6.2.2	Compressive strength.....	- 154 -
6.2.3	Strength activity index	- 160 -
6.2.4	Drying shrinkage.....	- 161 -
6.3	Effects of different types and levels of SCMs on fresh and hardened properties of mortar	- 164 -
6.3.1	Flow and wet density	- 164 -
6.3.2	Compressive strength.....	- 166 -
6.3.3	Strength activity index	- 171 -
6.3.4	Drying shrinkage.....	- 172 -
6.4	Summary	- 173 -
7	<i>EFFECT OF RUN-OF-STATION FLY ASH AND OTHER SCMS ON VARIOUS PROPERTIES OF LEAN AND HIGH-PERFORMANCE CONCRETE.....</i>	- 177 -
7.1	Preface	- 177 -
7.2	Effect of RFA and CFA on plastic and hardened properties in lean concrete mixes	- 177 -
7.2.1	Plastic concrete properties.....	- 180 -
7.2.2	Hardened concrete properties.....	- 181 -
7.2.3	Possible use of RFA replacing CFA in the lean concrete mix...	- 185 -

7.3	Effect of RFA and other SCMs on plastic and hardened properties of pavement concrete mixes.....	- 187 -
7.3.1	Plastic concrete properties.....	- 190 -
7.3.2	Hardened concrete properties.....	- 191 -
7.3.3	Possible use of RFA or SL replacing CFA in the pavement concrete mix.....	- 197 -
7.4	Effect of RFA and other SCMs on plastic, hardened and durability properties of bridge concrete	- 199 -
7.4.1	Plastic concrete properties.....	- 202 -
7.4.2	Hardened concrete properties.....	- 203 -
7.4.3	Durability properties	- 208 -
7.4.4	Possible use of RFA or SL or MK replacing CFA in bridge concrete mix.....	- 221 -
7.5	Summary	- 225 -
8	<i>CONCLUSIONS AND RECOMMENDATIONS</i>.....	- 228 -
8.1	Conclusions.....	- 228 -
8.2	Recommendations.....	- 232 -
8.3	Future work.....	- 234 -
	<i>Appendix A</i>	- 235 -
	<i>Appendix B</i>	- 287 -
	<i>References</i>	- 292 -

LIST OF FIGURES

Figure 2-1: Degree of hydration of C_3S in cement clinker, slag, and fly ash in blended cement [16] - 16 -

Figure 2-2: Effect of MK content on the consistency of concretes [31] - 19 -

Figure 2-3: Common cement heat evolution curve [34]..... - 21 -

Figure 2-4: Maximum heat evolution rate versus PC replacement level for binary PC–FA blends [36] - 22 -

Figure 2-5: Cumulative heat of hydration of PC–FA blends at 120 h relative to PC [36]..... - 23 -

Figure 2-6: Heat evolution curves for cement pastes with 30% fly ash replacement [37] - 24 -

Figure 2-7: Heat evolved within 24 and 72 hours of hydration [37] - 24 -

Figure 2-8: Heat rate profiles for the concrete containing cement and GGBFS [38]..... - 25 -

Figure 2-9: Heat production rate $mW\ g^{-1}$ in function of age under isothermal conditions (10 °C, 20 °C and 35 °C) for pastes with slag-to binder ratios of 0, 30, 50 or 85% [40]..... - 27 -

Figure 2-10: Maximum heat evolution rate versus PC replacement level for binary PC–MK blends [36]..... - 28 -

Figure 2-11: Cumulative heat of hydration of PC–FA blends at 120 h relative to PC [36]..... - 29 -

Figure 2-12: Average pore diameters of pastes at 28, 60 and 90 days [45]	- 30 -
Figure 2-13: Gel pores of pastes at 7, 28, 60 and 90 days [45]	- 30 -
Figure 2-14: XRD patterns of PC, 40OFA and 40CFA pastes at 90 days [46]	- 31 -
Figure 2-15: Pore size distribution versus curing time for a) radii <math>< 20\mu\text{m}</math> and b) radii > of $\mu\text{m}</math> pastes [52]$	- 35 -
Figure 2-16: Average pore diameter of blended cement pastes [25]	- 36 -
Figure 2-17: Total porosity of high-performance concrete [54]	- 37 -
Figure 2-18: Flexural strength development versus age [56]	- 38 -
Figure 2-19: Compressive strength development curves versus age [58]	- 39 -
-	-
Figure 2-20: Effect of GGBFS on compressive strength development [59]	- 40 -
Figure 2-21: Effect of GGBS on flexural strength at 90 days [59]	- 41 -
Figure 2-22: Effectiveness of MK on 28-day properties of concrete [62]	- 43 -
Figure 2-23: Effectiveness of MK in concrete - function of age and w/b ratio [54]	- 43 -
Figure 2-24: Test results of drying shrinkage of concrete [65]	- 45 -
Figure 2-25: Effect of metakaolin on drying shrinkage [66]	- 46 -

Figure 2-26: Effect of increasing GGBFS levels on chloride concentration of concrete [80].....	- 51 -
Figure 2-27: Chloride permeability of MK concretes [24].....	- 51 -
Figure 2-28: Chloride permeability of control and MK concretes [54] .	- 52 -
Figure 2-29: An accelerated test set-up for rapid sulfate permeability determination [90].....	- 55 -
Figure 2-30: Sulfate expansion of MK concretes [95]	- 57 -
Figure 2-31: Compressive strength reduction of MK concretes after 18 months in sodium sulfate solution[95]	- 58 -
Figure 2-32: Effect of ash composition and replacement level on expansion due to ASR [102]	- 60 -
Figure 2-33: ASR expansion (a) effect of GGBFS (b) effect of fineness [107].....	- 62 -
Figure 2-34: Alkali-silica expansion in MK concrete prisms [108]	- 63 -
Figure 2-35: Hydroxyl ion concentration of pore solutions expressed from pastes containing MK [108].....	- 64 -
Figure 3-1: Abbreviations used to denote paste mixes	- 79 -
Figure 3-2: Abbreviations used to denote mortar mixes	- 81 -
Figure 3-3: Abbreviations used to denote concrete mixes	- 85 -
Figure 3-4: Normal distribution of concrete strengths [129].....	- 88 -
Figure 3-5: Relationship between standard deviation and characteristic strength [129].....	- 88 -

Figure 3-6: Relationship between compressive strength and free W/C ratio [129].....	- 90 -
Figure 3-7: Recommended proportions of fine aggregate according to percentage passing 600 μm sieve [129].....	- 94 -
Figure 3-8: Recommended combined aggregates according to Road Note No.4.....	- 95 -
Figure 4-1: Particle size distribution of cement and fly ashes using the laser diffraction technique	- 102 -
Figure 4-2: Particle size distribution of cement, slag and metakaolin using the laser diffraction technique	- 102 -
Figure 4-3: Comparison of main oxide composition of cementitious materials using XRF analysis	- 104 -
Figure 4-4: $\text{CaO-Al}_2\text{O}_3\text{-SiO}_2$ ternary diagram of cementitious materials [139].....	- 105 -
Figure 4-5: X-ray diffraction patterns of CFA, RFA, GRFA (Q = quartz, M = mullite).....	- 107 -
Figure 4-6: X-ray diffraction of Slag (G = gypsum, B= Bassinite).....	- 108 -
Figure 4-7: X-ray diffraction of Metakaolin (Q = quartz).....	- 109 -
Figure 4-8: SEM micrograph of classified fly ash (CFA) (x 300)	- 110 -
Figure 4-9: SEM micrograph of run-of-station fly ash (RFA) (x 300)	- 110 -
Figure 4-10: SEM micrograph of ground run-of-station fly ash (GRFA) (x 300)	- 111 -

Figure 4-11: SEM micrograph of slag (x 300)	- 111 -
Figure 4-12: SEM micrograph of metakaolin (x 300)	- 112 -
Figure 4-13: Particle size grading of Sydney fine sand (sieving method)	- 114 -
Figure 4-14: Particle size grading of Nepean coarse sand (sieving method)	- 114 -
Figure 4-15: Particle size grading of 10 mm aggregate (sieving method)	- 115 -
Figure 4-16: Particle size grading of 20 mm aggregate (sieving method)	- 116 -
Figure 5-1: Common cement heat evolution curve [34].....	- 120 -
Figure 5-2: Effects of levels of different fly ash on heat evolution at w/b=0.40 (a=GRFA, b=CFA and c=RFA)	- 123 -
Figure 5-3: Effects of levels of different fly ash on heat evolution at w/b=0.55 (a=GRFA, b=CFA and c=RFA)	- 124 -
Figure 5-4: Effect of fineness of fly ash on heat evolution at w/b=0.40 (a=20%, b=30% and c=40% fly ash content)	- 127 -
Figure 5-5: Effect of fineness of fly ash on heat evolution at w/b=0.55 (a=20%, b=30% and c=40% fly ash content)	- 128 -
Figure 5-6: Effect of fineness and level of fly ash on cumulative heat evolution	- 130 -
Figure 5-7: Heat evolution of blended cement pastes containing slag .	- 132 -

Figure 5-8: Cumulative heat evolution of blended cement pastes containing slag	- 133 -
Figure 5-9: Heat evolution of blended cement pastes containing metakaolin (a= w/b:0.40 and b= w/b:0.55)	- 135 -
Figure 5-10: Cumulative heat evolution of the blended cement pastes containing metakaolin.....	- 136 -
Figure 5-11: XRD patterns of cement and 20% blended pastes at 7 days (P=portlandite).....	- 137 -
Figure 5-12: XRD patterns of cement and blended pastes at 28 days (P=portlandite).....	- 138 -
Figure 5-13: SEM micrographs of blended cement pastes with 20% fly ash at 1 day (x 3.00 K)	- 140 -
Figure 5-14: SEM micrographs of blended cement pastes with 20% fly ash at 7 days (x 3.00 K).....	- 143 -
Figure 5-15: SEM micrographs of blended cement pastes with 20% fly ash at 28 days (x 3.00 K).....	- 144 -
Figure 5-16: Flow of the pastes containing fly ashes with different fineness and cement replacement levels	- 146 -
Figure 5-17: Compressive strength development of the blended cement pastes containing 20% fly ash content of different fineness	- 147 -
Figure 5-18: Compressive strength development of the blended cement pastes containing 40% fly ash content of different fineness	- 147 -

Figure 5-19: Relative strength of the blended cement pastes containing varying fly ash content of different fineness	- 148 -
Figure 6-1: Flow of mortar mixes with fly ash replacement	- 153 -
Figure 6-2: Wet density of mortar mixes with fly ash replacement	- 154 -
Figure 6-3: Relative strength of high strength mortar with different cement replacement levels of fly ash at w/b=0.40 and s/b=2.5	- 159 -
Figure 6-4: Relative strength of low strength mortar with different cement replacement levels with CFA and RFA at w/b=0.55 and s/b=5.0	- 160 -
Figure 6-5: Strength activity index of mortar mixes with fly ash replacement	- 161 -
Figure 6-6: Flow of mortar mixes with different cement replacement levels of SCMs	- 165 -
Figure 6-7: Wet density of mortar mixes with different cement replacement levels of SCMs	- 165 -
Figure 6-8: Relative strength of mortar mixes with different cement replacement levels of SCMs (a=CFA, b=SL and c=MK)	- 170 -
Figure 6-9: Strength activity index of mortar mixes with different cement replacement levels of SCMs	- 171 -
Figure 7-1: Combined aggregate grading for C12-0, C12-CFA60 and C12-RFA60 mixes according to Road Note No.4	- 180 -
Figure 7-2: Compressive strength development in lean concrete mixes	- 183 -

Figure 7-3: Relative strength in lean concrete mixes	- 183 -
Figure 7-4: Drying shrinkage in lean concrete mixes.....	- 185 -
Figure 7-5: Combined aggregate grading for C40-C0, C40-CFA20, C40-RFA20 and C40-SL50 mixes according to Road Note No.4	- 190 -
Figure 7-6: Compressive strength development in pavement concrete mixes containing SCMs.....	- 193 -
Figure 7-7: Relative strength in pavement concrete mixes	- 194 -
Figure 7-8: Flexural strength development in pavement concrete mixes	- 195 -
Figure 7-9: Drying shrinkage in pavement concrete mixes up to 56 days	- 197 -
Figure 7-10: Combined aggregate grading for C50-C0, C50-CFA25, C50-RFA25, C50-SL50 and C50-MK15 mixes according to Road Note No.4	- 202 -
Figure 7-11: Relative strength in bridge concrete mixes.....	- 205 -
Figure 7-12: Compressive strength development in bridge concrete mixes	- 206 -
Figure 7-13: Drying shrinkage in bridge concrete mixes	- 207 -
Figure 7-14: Water absorption and AVPV percentage at 28 days according to AS 1012.21	- 210 -
Figure 7-15: Charge passed measured at 28 days in various concrete mixes using RCPT test	- 211 -

Figure 7-16: Rapid sulfate permeability of various concrete mixes at 28 days	- 215 -
Figure 7-17: Effect of CFA levels on average ASR expansion of mortar bars.....	- 218 -
Figure 7-18: Effect of RFA levels on average ASR expansion of mortar bars.....	- 218 -
Figure 7-19: Effect of slag levels on average ASR expansion of mortar bars	- 219 -
Figure 7-20: Effect of metakaolin levels on average ASR expansion of mortar bars	- 220 -

LIST OF TABLES

Table 2-1: Chemical composition of fly ash by coal type [13] - 10 -

Table 2-2: Physical properties of fly ash [15] - 11 -

Table 2-3: Typical chemical composition of slag [17]..... - 12 -

Table 2-4: Physical properties of slag [17]..... - 13 -

Table 2-5: Physical properties of metakaolin [4] - 14 -

Table 2-6: Typical chemical composition of metakaolin [4] - 14 -

Table 2-7: Summary of the test results for pore structure of mortar [49]- 33 -

-

Table 2-8: The influence of SCMs on the percentage of the volume of pores in the size range of mesopores and macropores [49]..... - 33 -

Table 2-9: Effect of metakaolin content on the compressive and flexural strength of concrete [62] - 42 -

Table 2-10: Mix proportioning (kg/**m³**) [65]..... - 45 -

Table 2-11: MIP measured the total porosity of concrete with w/b=0.3 [54] - 52 -

Table 2-12: Variation of compressive strength due to the sulfate attack [94]..... -56 -

Table 3-1: Plan for determining the effectiveness of fineness and levels of SCMs on the heat of hydration - 71 -

Table 3-2: Plan for determining optimum binary binder systems for mortars - 73 -

Table 3-3: Plan for determining effectiveness level for mitigation ASR- 73 -

Table 3-4: Mix design composition for high strength mortar	- 82 -
Table 3-5: Mix design composition for low strength mortar.....	- 82 -
Table 3-6: Mixing procedures used for mortar mixes	- 83 -
Table 3-7: K constant value based on defective percentage [129]	- 86 -
Table 3-8: Approximate compressive strength of concrete mixes made with free W/C ratio of 0.50 [129]	- 90 -
Table 3-9: Approximate free water contents required to give various levels of workability [129].....	- 92 -
Table 4-1: Specific surface area of cement and SCMs using Blaine's air permeability method	- 100 -
Table 4-2: Fineness of cement and SCMs passing 45- μ m sieve	- 101 -
Table 4-3: Specific gravity of cement and SCMs.....	- 101 -
Table 4-4: Oxide composition of cementitious materials using XRF analysis	- 104 -
Table 4-5: Alkali and available alkali content of SCMs	- 105 -
Table 4-6: Comparison of fly ashes with the specified requirement of Australian Standard AS3582.1 [6].....	- 106 -
Table 4-7: Sieve analysis for fine and coarse sands	- 113 -
Table 4-8: Sieve analysis for 10 and 20 mm coarse aggregates	- 115 -
Table 4-9: Fine and coarse aggregate properties	- 116 -
Table 6-1: Compressive strength of mortar mixes (MPa) with w/b=0.4 and S/b=2.5.....	- 157 -

Table 6-2: Compressive strength of mortar mixes (MPa) with w/b=0.55 and S/b=5.0.....	- 157 -
Table 6-3: Drying shrinkage of high strength mortar mixes at different ages with different cement replacement levels of fly ash (w/b=0.40, s/b=2.5)	- 163 -
Table 6-4: Drying shrinkage of low strength mortar mixes at different ages with different cement replacement levels of fly ash (w/b=0.55, s/b=5.0)	- 163 -
Table 6-5: Compressive strength of mortar mixes with different cement replacement levels of SCMs at different ages (MPa)	- 170 -
Table 6-6: Drying shrinkage of mortar mixes at different ages with different cement replacement levels of SCMs.....	- 173 -
Table 7-1: Mix design composition for lean concrete mixes	- 178 -
Table 7-2: Plastic and hardened concrete property requirements for lean mix according to RMS R82 specification [110].....	- 179 -
Table 7-3: Plastic property results for lean mixes	- 181 -
Table 7-4: Mix design composition for pavement concrete mixes	- 188 -
Table 7-5: plastic and hardened concrete property requirements for pavement concrete mix according to RMS R83 specification [111]....	- 189 -
Table 7-6: plastic property results for pavement concrete mixes	- 191 -
Table 7-7: Compressive strength results of pavement concrete mixes up to 56 days	- 194 -

Table 7-8: Mix design composition for bridge concrete mixes.....	- 200 -
Table 7-9: Plastic and hardened concrete property requirements for bridge concrete mix according to RMS B80 specification [112]	- 201 -
Table 7-10: Plastic property results for bridge concrete mixes	- 203 -
Table 7-11: Charge passed for individual specimens measured at 28 days in various concrete mixes using RCPT test	- 211 -
Table 7-12: Chloride ion penetration based on charge passes according to ASTM C1202.....	- 212 -
Table 7-13: Charge passed for individual specimens measured at 28 days using rapid sulfate permeability test	- 215 -
Table 7-14: Alkali, available alkali and Ca/Si ratio of SCMs	- 221 -
Table A-1: Step 1 in designing the lean concrete mix (C12-C0)	- 235 -
Table A-2: Step 2 in designing the lean concrete mix (C12-C0)	- 236 -
Table A-3: Step 3 in designing the lean concrete mix (C12-C0)	- 236 -
Table A-4: Step 4 in designing the lean concrete mix (C12-C0)	- 237 -
Table A-5: Step 5 in designing the lean concrete mix (C12-C0)	- 238 -
Table A-6: Final lean concrete mix (C12-C0).....	- 239 -
Table A-7: Step 1 in designing the lean concrete mix (C12-CFA60) ..	- 240 -
Table A-8: Step 2 in designing the lean concrete mix (C12-CFA60) ..	- 241 -
Table A-9: Step 3 in designing the lean concrete mix (C12-CFA60) ..	- 241 -
Table A-10: Step 4 in designing the lean concrete mix (C12-CFA60)	- 242 -
Table A-11: Step 5 in designing the lean concrete mix (C12-CFA60)	- 243 -

Table A-12: Final lean concrete mix (C12-CFA60).....	- 244 -
Table A-13: Step 1 in designing the lean concrete mix (C12-RFA60)	- 245 -
Table A-14: Step 2 in designing the lean concrete mix (C12-RFA60)	- 246 -
Table A-15: Step 3 in designing the lean concrete mix (C12-RFA60)	- 246 -
Table A-16: Step 4 in designing the lean concrete mix (C12-RFA60)	- 247 -
Table A-17: Step 5 in designing the lean concrete mix (C12-RFA60)	- 248 -
Table A-18: Final lean concrete mix (C12-RFA60).....	- 249 -
Table A-19: Step 1 in designing the pavement concrete mix (C40-C0)-	250
-	
Table A-20: Step 2 in designing the pavement concrete mix (C40-C0)-	251
-	
Table A-21: Step 3 in designing the pavement concrete mix (C40-C0)-	251
-	
Table A-22: Step 4 in designing the pavement concrete mix (C40-C0)-	252
-	
Table A-23: Step 5 in designing the pavement concrete mix (C40-C0)-	253
-	
Table A-24: Final pavement concrete mix design (C40-C0)	- 254 -
Table A-25: Step 1 in designing the pavement concrete mix (C40-CFA20 and C40-RFA20).....	- 255 -
Table A-26: Step 2 in designing the pavement concrete mix (C40-CFA20 and C40-RFA20).....	- 256 -

Table A-27: Step 3 in designing the pavement concrete mix (C40-CFA20 and C40-RFA20).....	- 256 -
Table A-28: Step 4 in designing the pavement concrete mix (C40-CFA20 and C40-RFA20).....	- 257 -
Table A-29: Step 5 in designing the pavement concrete mix (C40-CFA20 and C40-RFA20).....	- 258 -
Table A-30: Final pavement concrete mix design (C40-CFA20)	- 259 -
Table A-31: Final pavement concrete mix design (C40-RFA20)	- 260 -
Table A-32: Step 1 in designing the pavement concrete mix (C40-SL50)	- 261 -
Table A-33: Step 2 in designing the pavement concrete mix (C40-SL50)	- 262 -
Table A-34: Step 3 in designing the pavement concrete mix (C40-SL50)	- 262 -
Table A-35: Step 4 in designing the pavement concrete mix (C40-SL50)....	- 263 -
Table A-36: Step 5 in designing the pavement concrete mix (C40-SL50)....	- 264 -
Table A-37: Final pavement concrete mix design (C40-SL50)	- 265 -
Table A-38: Step 1 in designing the bridge concrete mix (C50-C0)....	- 266 -
Table A-39: Step 2 in designing the bridge concrete mix (C50-C0)....	- 267 -
Table A-40: Step 3 in designing the bridge concrete mix (C50-C0)....	- 267 -

Table A-41: Step 4 in designing the bridge concrete mix (C50-C0)....	- 268 -
Table A-42: Step 5 in designing the bridge concrete mix (C50-C0)....	- 269 -
Table A-43: Final bridge concrete mix design (C50-C0).....	- 270 -
Table A-44: Step 1 in designing the bridge concrete mix (C50-CFA25 & C50-RFA25)	- 271 -
Table A-45: Step 2 in designing the bridge concrete mix (C50-CFA25 & C50-RFA25)	- 272 -
Table A-46: Step 3 in designing the bridge concrete mix (C50-CFA25 & C50-RFA25)	- 272 -
Table A-47: Step 4 in designing the bridge concrete mix (C50-CFA25 & C50-RFA25)	- 273 -
Table A-48: Step 5 in designing the bridge concrete mix (C50-CFA25 & C50-RFA25)	- 274 -
Table A-49: Final bridge concrete mix design (C50-CFA25).....	- 275 -
Table A-50: Final bridge concrete mix design (C50-RFA25).....	- 276 -
Table A-51: Step 1 in designing the pavement concrete mix (C50-MK15)...	- 277 -
Table A-52: Step 2 in designing the pavement concrete mix (C50-MK15)...	- 278 -
Table A-53: Step 3 in designing the pavement concrete mix (C50-MK15)...	- 278 -

Table A-54: Step 4 in designing the pavement concrete mix (C50-MK15)...	- 279 -
Table A-55: Step 5 in designing the pavement concrete mix (C50-MK15)	-280 -
Table A-56: Final bridge concrete mix design (C50-MK15).....	- 281 -
Table A-57: Step 1 in designing the bridge concrete mix (C50-SL50)	- 282 -
Table A-58: Step 2 in designing the bridge concrete mix (C50-SL50)	- 283 -
Table A-59: Step 3 in designing the bridge concrete mix (C50-SL50)	- 283 -
Table A-60: Step 4 in designing the bridge concrete mix (C50-SL50)	- 284 -
Table A-61: Step 5 in designing the bridge concrete mix (C50-SL50)	- 285 -
Table A-62: Final bridge concrete mix design (C50-SL50).....	- 286 -
Table B-1: Compressive strength results of lean concrete mixes (MPa).....	- 287 -
Table B-2: Compressive strength results of pavement concrete mixes (MPa)	- 287 -
Table B-3: Compressive strength results of bridge concrete mixes (MPa)	- 288 -
Table B-4: Flexural strength results of pavement concrete mixes (MPa).....	- 288 -
Table B-5: Drying shrinkage results of lean concrete mixes.....	- 288 -
Table B-6: Drying shrinkage results of pavement concrete mixes.....	- 289 -

Table B-7: Drying shrinkage results of bridge concrete mixes - 289 -

Table B-8: Expansion of mortar bars due to the alkali-silica reactivity- 290 -

ABBREVIATIONS

AMBT	Accelerated Mortar Bar Test
AEA	Air Entraining Agent
ASR	Alkali-Silica Reactivity
ACI	American Concrete Institute
AVPV	Apparent Volume of Permeable Void
AS	Australian Standard
CH	Calcium Hydroxide
C-S-H	Calcium Silicate Hydrate
CCAA	Cement Concrete & Aggregate Australia organisation
CFA	Classified Fly Ash
ESP	Electrostatic Precipitators
FF	Fabric Filter
FA	Fly Ash
GGBFS	Ground Granulated Blast Furnace Slag
GRFA	Grounded Run-of-Station Fly Ash
HPC	High-Performance Concrete
HWR	High-range Water Reducer
LOI	Loss-Of-Ignition
MIP	Mercury Intrusion Porosimetry
MK	Metakaolin
MAU	Microstructural Analysis Unit
OFA	Original Fly Ash
PSA	Particle Size Analysis
PC	Portland Cement
RCPT	Rapid Chloride Permeability Test
RH	Relative Humidity
RMS	Road and Maritime Services
RFA	Run-of-the-Station Fly Ash
SSD	Saturated-Surface-Dry
SEM	Scanning Electron Microscopy
SL	Slag
SAI	Strength Activity Index Test
SCMs	Supplementary Cementitious Materials
C ₃ A	Tricalcium Aluminate

UNSW	University of New South Wales
UTS	University of Technology Sydney
WR	Water Reducer
XRD	X-ray Diffraction
XRF	X-ray Fluorescence

ABSTRACT

The utilization of supplementary cementitious materials (SCMs) for the partial replacement of cement in concrete introduces several environmental and economical benefits. Some of the major benefits include the conservation of natural resources, the reduction of greenhouse gas emissions and a reduction in cement use. The use of SCMs in concrete also have technical merit and are multifaceted in their approach by enhancing the workability, strength and durability of concrete. In recent years, Australian road authorities have been increasingly concerned with the shortage of classified fly ash (CFA) predicted to occur in the near future with renewable energies finding their way into the market. Finding alternative SCMs is a critical issue due to a shortage of national resources available. Run-of-station fly ash (RFA) can be a possible alternative to CFA. However, the lack of enough information available in the literature hinders the use of RFA, and additional work needs to be carried out to investigate its efficiency in different concrete mixes.

In this investigation, an experimental study has been carried out to evaluate the effect of fineness of three fly ashes (ground run-of-station fly ash (GRFA), RFA and CFA) on the heat of hydration behaviour, flow and compressive strength properties of blended cement pastes. The study has been extended to also assess the influence of fineness, types and levels of SCMs (fly ash, slag or metakaolin) on the fresh and hardened properties of mortars. Finally, the effect of RFA and other SCMs on the fresh, hardened and durability properties of concretes for different applications such as lean, pavement and bridge work have also been studied.

Among all the SCMs investigated, metakaolin (MK) was found to have the highest fineness followed by slag (SL), GRFA, CFA and RFA. In addition, increasing the fineness of fly ash showed a reduction in the crystallinity of silica particles, which resulted in more reactivity suggesting possibly more amorphous silica particles present for pozzolanic reactivity. The presence of smaller quantities of crystalline silica particles in SL and MK also suggests a higher degree of pozzolanic reactivity may prevail. Results of the study on the blended pastes showed that the cumulative heat of hydration decreased by the incorporation of fly ash with different fineness (302, 368 and 495 m²/kg), SL, or MK. This reduction was noted to increase with increasing SCM content. The finer grade fly ash (GRFA) generated higher heat of hydration compared to coarser grade fly ash (RFA). Furthermore, the consumption of portlandite increased with increasing fineness of fly ash at 28 days reflecting higher pozzolanic reactivity resulting in higher compressive strength than the blended pastes containing coarser grade fly ash.

Partially replacing cement with 20%, 30% and 40% fly ash of different fineness (CFA, RFA and GRFA), and 35%, 50% and 65% SL in mortars (using a fixed w/b ratio) improved the flow compared to control mortar (devoid of SCM addition). The inclusion of 5%, 10% and 15% MK in the same mortar decreased the flow compared to control mortar. Increasing the fineness of fly ash from 302 to 495 m²/kg improved the 28-day compressive strength of the mortar even to a similar level to the control mortar. However, the 28-day compressive strength of mortars containing SL and MK were even higher than control mortar at all replacement levels evaluated. The 56-day drying shrinkage of mortars was also

found to decrease considerably when partially replacing cement with SCMs. Compared to the control mortar, a reduction in drying shrinkage was noted with an increase in SCM replacement level.

Results from the study on lean concretes with 60% fly ash content revealed RFA is almost as effective as CFA in satisfying the strength and drying shrinkage requirements according to the RMS R82 specification despite having lower compressive strength development at all ages (up to 56 days) compared to the same concrete containing CFA. Replacing cement with 20% and 25% RFA in pavement and bridge concretes decreased 28-day compressive strength by about 11% and 11% respectively, compared to the same concretes containing CFA. In addition, there was a slight increase in drying shrinkage at all ages (up to 56 days) by replacing cement with RFA instead of CFA in all concretes investigated. It can be concluded that the concretes with RFA can achieve similar hardened properties to CFA concrete in pavement concrete and bridge applications by only adjusting the mix design such as lowering w/b ratio. This statement is based on the results of fineness level, crystalline phase identification and oxide composition of CFA and RFA particles and also strength development from the pastes and mortars. Moreover, partially replacing cement with SL or MK could be another alternative to CFA in pavement concrete and bridge applications due to the high fineness confirmed by mortar and concrete results. However, only the relative performance of concrete used for bridge applications was assessed for durability properties.

According to the qualitative tests carried out on the bridge concretes containing SCMs, it was noted that the level of penetrability of chloride and sulfate ions into the concrete was reduced by replacing cement with 25% CFA or

RFA compared to the control concrete at 28 days. This reduction in penetrability level was significant for the concretes containing 50% SL or 15% MK. In addition, the results from the accelerated mortar bar test revealed that partially replacing cement with SCMs decreased the expansion of mortar bars due to alkali-silica reaction (ASR); the reduction in expansion increased with an increase in SCM content. For the reactive aggregate investigated in this study, incorporation of 25% CFA or 25% RFA or 50% SL or 15% MK is needed to control the expansion of mortar bars due to ASR.

This experimental investigation has demonstrated the value and benefits of using RFA as an alternative SCM in concrete. RFA has proven abilities to achieve the same level of performance as CFA when used in various concrete applications such as lean, pavement and bridge concrete mix designs. SL and MK could also be used as other alternatives to CFA in concrete.