

Adverse Effects of Expansive Soils on Road Infrastructure and Evaluation of Chemical Remediation Techniques

by Hayder Abbas Hasan

Thesis submitted in fulfilment of the requirements for the degree of **Doctor of Philosophy**

under the supervision of A. Prof. Hadi Khabbaz and A. Prof. Behzad Fatahi

University of Technology Sydney Faculty of Engineering and Information Technology January 2019

CERTIFICATE OF ORIGINAL AUTHORSHIP

I, Hayder Abbas Hasan declare that this thesis is submitted in fulfilment of the requirements for the award of Doctor of Philosophy, in the school of Civil and Environmental Engineering, Faculty of Engineering and Information Technology at the University of Technology Sydney.

This thesis is wholly my own work unless otherwise reference or acknowledged. In addition, I certify that all information sources and literature used are indicated in the thesis. This document has not been submitted for qualifications at any other academic institution. This research is supported by the Australian Government Research Training Program.

Production Note:Signature:Signature removed prior to publication.

Date:

20/11/2019

Abstract

Using waste materials and creating novel methods to minimise the adverse effects of expansive soils reduces the cost of building foundations and also reduces their impact on the environment and landfill. Continuous swelling and shrinking cause cracks to form in structures built on expansive soils such as road pavements and embankments. This study describes how a combination of lime and sugarcane bagasse ash, a waste product of the sugar industry, is used to stabilise expansive soils. Since the amount of research addressing the effect of bagasse ash on soil stabilisation was limited, a comprehensive attempt has been made on expansive soils using bagasse ash to improve their engineering properties and reduce the need for traditional additives such as lime. There are large gaps with regards to the optimum amount of bagasse ash that can be used for lime treated soil, the most effective ratio between bagasse and the amount of lime, and the properties of soil treated with bagasse ash, and with or without lime. An array of tests to quantify the shear strength of treated soils, the durability of treated soils using different drying and wetting cycles, and the soil water characteristic curves of treated soils were carried out.

Two types of expansive soils were used in this experimental program; this first type came from a road construction site in Queensland (Soil Q) and is classified as a high plasticity clay; the second type was created in the laboratory by mixing 80% kaolin and 20% bentonite (Soil C). It was also defined as high plasticity clay. These samples were prepared by integrating soil with a mixture of hydrated lime (L) from 0 to 6.25% and up to 25% of bagasse ash. These combinations of lime to bagasse ash were prepared with different ratios (e.g. 1:1, 1:2, 1:3 and 1:4). A combination of lime and bagasse ash was added to the soil samples as a stabiliser at ratios of 6%, 10% 18% and 25%, based on the dry weight of soil. To determine how hydrated lime and bagasse ash affects the engineering properties of treated soils, tests such as particle size distribution, specific gravity, Atterberg limits, compaction, linear shrinkage, the free swell ratio, unconfined compressive strength (UCS), durability, the California bearing ratio

(CBR), free swell, swell pressure, consolidation, and direct shear and suction tests were carried out.

The results indicate that the dry density of natural and artificial soils decreases as the amount of additives increase, and the linear shrinkage, free swell ratio, free swelling and swell pressure of soils treated with a combination of lime and bagasse ash decreased more than soils treated only with lime or bagasse ash. The UCS and CBR of soil increased as the amount of additives and curing time increased, and when bagasse ash was added to soil treated with lime under various cycles of wetting-drying, it became more durable than soil treated just with lime. Moreover, the addition of bagasse ash to soil treated with lime reduced the gap between soaked and unsoaked CBR. The pre-consolidation pressure (σ_c'), the coefficient of consolidation (C_v), the compression index (C_c), the swell index (C_s), and the cohesion and friction angle of soils improved as the amount of additives (bagasse ash and/or lime) increased. The matric suction was determined based on the gravimetric water content and degree of saturation, using the filter paper technique. The air entry value parameter (a) of soils increased after adding 6.25% lime, but the highest a value was for soil samples treated with 6.25% L and 18.75% BA.

PLAXIS software was used to calculate the deformation and to evaluate the slope stability of a proposed embankment placed on top of the target expansive soil (Soil Q) under a fill embankment. Five models were developed to estimate the settlement of soil and assess the slope stability of a fill embankment under traffic loads. The Mohr-Coulomb model was used to simulate a fill embankment as well as treated soil and dense sand, while a soft soil model was applied to untreated soil with an over consolidation ratio of 1.2. These numerical models indicated that treating soil reduced its vertical displacement (settlement) due to an increase in its strength parameters, whereas the reduction in time is associated with an increase in its permeability. The slope stability of the fill embankment decreased slightly as the layer of treated soil was made thicker.

This study offers a promising way of using bagasse ash as an eco-friendly stabiliser for treating expansive soils, particularly in conjunction with lime for treatment of weak subgrade of roads.

Acknowledgements

This PhD project could not have been possible without the support provided by numerous people. In particular, I would like to express my deepest appreciation and gratitude to my supervisor, Associate Professor Hadi Khabbaz, for his excellent guidance and enthusiastic encouragement throughout this research study. His dedication and immeasurable efforts towards the field of Geotechnical Engineering always inspire me. In addition, his support, patience and most of all his friendship throughout this research are truly cherished. I would also like to thank my co-supervisor Associate Professor Behzad Fatahi, for his unfailing assistance, guidance and support over the past few years.

I wish to thank the technical, the administrative and the support staff at school of civil and environmental engineering at the University of Technology Sydney, for providing technical guidance, laboratory supports and research facilities.

I would also like to express my gratitude to my friends and colleagues, who supported me with their valuable advice and assistance. I am grateful to my father, mother and brothers for instilling patience and wisdom needed to complete this PhD project. Finally, I would like to express my deepest love to my wife, Taqwa Al-Khafaji, for her unconditional support in good and bad times.

Dedication

I would like to dedicate this Doctoral dissertation to my family, particularly my son, Ali, and my daughter, Huda. They have made me stronger, better and more fulfilled than I could have ever imagined. I love them to the moon and back. I equally dedicate this thesis to my wife Taqwa Al-Khafaji for her love, support and understanding.

List of Publications

- Hasan, H., Khabbaz, H., & Fatahi, B. (2016). Impact of quicklime and fly ash on the geotechnical properties of expansive clay. *4th Geo-China International Conference 2016*, pp. 93-100.
- Hasan, H., Dang, L., Khabbaz, H., Fatahi, B., & Terzaghi, S. (2016). Remediation of Expansive Soils Using Agricultural Waste Bagasse Ash. *Procedia Engineering*, 143, pp.1368-1375.
- Hasan, H., Khabbaz, H., & Fatahi, B. (2016). Expansive Soil Stabilization Using Lime-Bagasse Ash. 69th Canadian Geotechnical Conference GeoVancouver 2016.
- Hasan, H., Khabbaz, H., & Fatahi, B. (2017). Strength Property of Expansive Soils Treated with Bagasse Ash and Lime. *Springer International Publishing, Sustainable Civil Infrastructures,* pp. 22-35: Innovative Infrastructure Geo-technology.
- Hasan, H., Dang, L., Khabbaz, H., & Fatahi, B. (2017). Swelling pressure and consolidation of soft clay stabilized with bagasse ash and lime. *Proceedings of the 10th International Conference on the Bearing Capacity of Roads, Railways and Airfields (BCRRA 2017)*, June 28-30, 2017, Athens, Greece: CRC Press.
- Dang, L.C., Hasan, H., Fatahi, H., Jones, R. & Khabbaz, H. (2016). Enhancing the Engineering Properties of Expansive Soil Using Bagasse Ash and Hydrated Lime. *International journal of GEOMATE*, vol. 11, no. 25, pp. 2447-2454.
- Dang, L., Hasan, H., Fatahi, B. & Khabbaz, H. 2015, 'Influence of Bagasse Ash and Hydrated Lime on Strength and Mechanical Behaviour of Stabilised Expansive Soil', *GEOQuébec 2015*, Québec City, Canada.
- Dang, L., Hasan, H., Fatahi, B., Jones, R. & Khabbaz, H. 2015. Effects of Bagasse Ash and Hydrated Lime Addition on Engineering Properties of Expansive Soil. *GEOMATE 2015*, The GEOMATE International Society, Osaka, Japan.

Table of Contents

Abstractii
Acknowledgements iv
List of Publications vi
List of Figures xii
List of Tablesxxii
Chapter 1. Introduction
1.1 Background1
1.2 Research Scope and Objectives
1.3 Significance of this Research
1.4 Outline of the Thesis
Chapter 2. Literature Review
2.1 Introduction
2.2 Factors Affecting the Behaviour of Expansive Soil
2.2.1 Change in Water Content
2.2.2 Mineralogical Feature of Clay
2.2.3 Change of the Effective Stress and Suction
2.2.4 Density and Water Content
2.3 Movement in Expansive Soil and Magnitude of Heave
2.3.1 The Cyclic of Heave and Shrinkage
2.3.2 Progressive Swelling beneath Centre of Foundation
2.4 Identification of Expansive Soil Based on Physical Properties
2.4.1 Soil Index Properties Methods
2.4.2 Free Swell Test

2.4.3	Standard Absorption Moisture Content (SAMC)	21
2.4.4	Coefficient of Linear Extensibility (COLE)	23
2.5 Soi	l Stabilisation	23
2.5.1	Lime Stabilisation	29
2.5.2	Fly Ash Stabilisation	35
2.5.3	Bagasse Ash Stabilisation	42
2.6 Sur	nmary	54
Chapter 3.	Materials and Methods	56
3.1 Intr	oduction	56
3.2 Ma	terials	56
3.2.1	Queensland soil (Soil Q)	57
3.2.2	Combination of kaolin - bentonite soil (Soil C)	57
3.2.3	Hydrated lime (L)	58
3.2.4	Bagasse ash (BA)	58
3.3 Soi	l Preparation and Mixing with Additives	59
3.3.1	Queensland soil (Soil Q)	59
3.3.2	Kaolin - bentonite soil (Soil C)	59
3.4 Exp	perimental Program	60
3.4.1	Particle size distribution	61
3.4.2	Specific gravity	61
3.4.3	Atterberg limits	61
3.4.4	Standard compaction	63
3.4.5	Linear shrinkage (LS)	69
3.4.6	Free swell ratio (FSR)	69
3.4.7	Unconfined compressive strength (UCS)	70

3.4	4.8	Durability	72
3.4	4.9	California bearing ratio (CBR)	74
3.4	4.10	Free swelling	75
3.4	4.11	Swell pressure and Consolidation	77
3.5	Sur	nmary	80
Chapter	r 4.	Results and Discussion of General Tests	82
4.1	Intr	oduction	82
4.2	Lin	ear Shrinkage Tests (LS)	83
4.2	2.1	Effect of the admixtures on Queensland soil (Soil Q)	83
4.2	2.2	Effect of the admixtures on Soil C	83
4.3	Fre	e Swell Ratio Tests (FSR)	85
4.3	3.1	Effect of the admixtures on FSR of Soil Q	85
4.3	3.2	Effect of the admixtures on FSR of Soil C	86
4.4	Uno	confined Compressive Strength Tests (UCS)	87
4.4	4.1	Effect of the admixtures on UCS of Soil Q	88
4.4	4.2	Effect of the admixtures on UCS of Soil C	92
4.5	Du	rability Tests (Drying – Wetting Cycles)	95
4.5	5.1	Effect of the admixtures on durability of Soil Q	95
4.5	5.2	Effect of the admixtures on durability of Soil C	. 102
4.6	Cal	ifornia Bearing Ratio (CBR) Tests	. 106
4.6	5.1	Effect of the admixtures on CBR of Soil Q	. 106
4.6	5.2	Effect of the admixtures on CBR of Soil C	. 113
4.7	Fre	e Swelling Tests (FS)	. 119
4.7	7.1	Effect of the admixtures on free swelling of Soil Q	. 119
4.7	7.2	Effect of the admixtures on free swelling of Soil C	. 123

4.8	Sw	ell pressure and Consolidation Tests	124
4	.8.1	Effect of admixture content on swell pressure and consolidation of Soil Q	2 124
4	.8.2	Effect of admixture content on swell pressure and consolidation of Soil C	2 138
4.9	Su	mmary	146
Chapt	er 5.	Shear Strength of Expansive Soils before and after Stabilisation	149
5.1	Int	roduction	149
5.2	The	e Principal Mechanism of Shearing Using Direct Shear Apparatus	150
5.3	Re	view of Chemical Treatment Using Direct Shear Test with Clayey Soils	151
5.4	Sar	nple Preparation and Test Procedure	158
5.5	Re	sults and Discussion	159
5	5.5.1	Queensland expansive soil (Soil Q)	160
5	5.5.2	Created soil a mixture of 80% kaolin and 20% bentonite (Soil C)	168
5.6	Su	mmary	175
Chapt	er 6.	Soil Water Characteristic Curves	177
6.1	Int	roduction	177
6.2	De	finition of Unsaturated Soil	177
6.3	Fu	ndamentals of Soil Water Characteristic Curve	178
6.4	Filt	ter Paper Method to Determine SWCC	179
6.5	Ma	thematical Equation of SWCC	181
6	5.5.1	Brooks and Corey (1964) model	182
6	5.5.2	van Genuchten (1980) model	182
6	5.5.3	Fredlund and Xing (1994) model	183
6	5.4	Pasha et al. (2015) model	183
6.6	Stu	dies Referring to the Effect of Additives on SWCC Properties	184
6.7	Sar	nple Preparation and Test Procedure	186

6.8 Results and Discussion	188
6.8.1 SWCC of treated and untreated Queensland soil (Soil Q)	188
6.8.2 SWCC of Soil C without treatment or with treatment	193
6.9 Summary	197
Chapter 7. Numerical Investigations on Performance of Road Embankments C Stabilised Expansive Soil)verlying 199
7.1 Introduction	199
7.2 Numerical Modelling of Embankments Overlying Expansive Soils	199
7.2.1 Model 1: Analysing the embankment behaviour without considering to of uplift pressure	the effect 201
7.2.2 Model 2: Analysing the embankment behaviour including the effect pressure	of uplift 213
7.2.3 Model 3: Comparing the embankment performance while the subg	rade soil
treated with lime or combined lime and bagasse ash	216
7.2.4 Model 4: Evaluating the effect of increasing the side slope angle embankment overlying untreated and treated soil using combined lib bagasse ash	e of the ime and 218
7.2.5 Model 5: Investigating the effect of geogrid inclusion at different lo	cation of
lime-bagasse ash treated soil on performance of the embankment	218
7.3 Summery	221
Chapter 8. Conclusions and Recommendations	223
8.1 Summary	223
8.2 Concluding Remarks	225
8.3 Recommendation for Further Study	236
References	237

List of Figures

Figure 2.13 (a) CBR values of various bentonite-lime mixtures with various sand contents
(b) UCS of bentonite - lime mixtures with various sand contents and curing time (Schanz and
Elsawy, 2017)
Figure 2.14 Effect of different lime (L) and fly ash (FA) contents (e.g. one L to three FA ratio)
on consistency of various expansive soils (Hasan et al.2016)
Figure 2.15 Effect of different lime (L) and fly ash (FA) contents (e.g. one L to three FA ratio)
on linear shrinkage of (40% B -60% K) soil after 3, 7 and 28 days of curing (Hasan et
al.2016)
Figure 2.16 Effect of different lime (L) and fly ash (FA) contents (e.g. one L to three FA ratio)
on:(a) Optimum moisture content (OMC), (b) maximum dry density (MDD) of two expansive
soils (Hasan et al.2016)
Figure 2.17 Effect of different lime (L) and fly ash (FA) contents (e.g. one L to three FA ratio)
on unconfined compressive strength (UCS) of (40% B -60% K) soil at various curing days
(Hasan et al.2016)
Figure 2.18 Effect of 3% lime – 9% fly ash on UCS of different soils at 3, 7 and 28 curing
days (Hasan et al.2016)
Figure 2.19 Effect of 5% lime – 15% fly ash on UCS of different soils at 3, 7 and 28 curing
days (Hasan et al.2016)
Figure 2.20 Swelling (%) versus time (min) of (40% B -60% K) soil (Hasan et al. 2016) 41
Figure 2.21 Swelling (%) versus time (min) of (40% B -60% K) soil treated different lime (L)
and fly ash (FA) contents (e.g. one L to three FA ratio) at 7 days of curing (Hasan et al.,
2016)
Figure 2.22 Bagasse ash landfill (Singh et al. 2018)
Figure 2.23 X-ray diffraction patterns of virgin bagasse ash: (a) after Julphunthong (2016),
and (b) after Madurwar et al. (2016)
Figure 2.24 SEM photographs of BA; (a) original magnification $\times 1,000$, and (b) original
magnification $\times 3,000$ after Julphunthong (2016), while (c) original magnification $\times 1,000$
after Jamsawang et al. (2017) 46
Figure 2.25 Effect of increasing bagasse ash content (%) on (a) soaked CBR, and (b) UCS
with different curing time (Anupam et al., 2017)

Figure 2.26 Effect of increasing bagasse ash content (%) on (a) Cohesion stress, and (b)
internal friction angle with different curing time (Anupam et al., 2017)
Figure 2.27 SEM of using 25% bagasse ash with soil (Anupam et al., 2017)
Figure 2.28 Relationship between strength loss and curing time with different replacement
of cement by bagasse ash (Jamsawang et al., 2017)
Figure 2.29 Relationship between the modulus of elasticity (E ₅₀) and the unconfined
compressive (q_u) (Jamsawang et al., 2017)
Figure 3.1 Bagasse ash after sieving on sieve 425 μm
Figure 3.2 Queensland soil (a) before air drying and (b) after sieving on 2.36 mm
Figure 3.3 Flowchart of testing program
Figure 3.4 Particle size distribution curve of Queensland soil (Soil Q)
Figure 3.5 Particle size distribution curve bagasse ash (BA)
Figure 3.6 Casagrande's device to determine the liquid limit of Soil Q before the test (soil
paste with a groove), and (b) after the test (groove closure) over 12 mm length
Figure 3.7 Standard compaction test results for Queensland soil (Soil Q)
Figure 3.8 Standard compaction test results for Soil C
Figure 3.9 Linear shrinkage moulds of Queensland soil after drying in the oven
Figure 3.10 Free swell ratio of Soil C treated with different additive contents
Figure 3.11 (a) Specimens of Soil C during the curing period, and (b) a specimen of Soil C
during UCS testing
Figure 3.12 Specimens of Queensland soil treated with 6.25% (a) air dry cycle No. 3 and (b)
<i>wet cycle No. 6.</i>
Figure 3.13 CBR specimens of Soil C before putting in plastic bags to be cured
Figure 3.14 Free swelling test of Soil Q treated with different additives under 1 kPa
surcharge
Figure 3.15 A specimen of Soil C treated with 6.25% L-18.75% BA after 7 days of curing for
swell pressure and consolidation tests
Figure 4.1 Effect of additives on liner shrinkage of Soil Q with different curing time 84
Figure 4.2 Influence of admixture content on free swell ratio (FSR) of control and treated
<i>Soil C</i> 87
Figure 4.3 Effect of variation of bagasse ash on UCS of Soil Q at 3,7 and 28 curing days.88

Figure 4.4 Effect of added lime on UCS of Soil Q at 3,7 and 28 curing days
Figure 4.5 Effect of added lime-bagasse ash on UCS of Soil Q at 3, 7 and 28 days of curing.
Figure 4.6 Comparison between UCS of Soil Q treated with L and soil treated with L-BA at
3 days of curing
Figure 4.7 Comparison between UCS of Soil Q treated with L and soil treated with L-BA at
7 days of curing
Figure 4.8 Comparison between UCS of Soil Q treated with L and soil treated with L-BA at
28 days of curing
Figure 4.9 Effect of BA on Soil C-2.5% L with different curing times
Figure 4.10 Effect of BA on Soil C-4.5% L with different curing times
Figure 4.11 Effect of BA on Soil C-6.25% L with different curing times
Figure 4.12 UCS verse lime (L) to bagasse ash (BA) ratio of Soil C treated with 6.25% L and
different BA contents ranging from 6.25% until to 31.25%
Figure 4.13 Stress-strain relationship of Soil Q treated with 6.25% L-18.75% BA after 2
drying-wetting cycles
Figure 4.14 Stress-strain relationship of Soil Q treated with 6.25% L after 2 drying-wetting
<i>cycles</i>
Figure 4.15 Stress-strain relationship of Soil Q treated with 6.25%L-18.75% BA after 3
drying-wetting cycles
Figure 4.16 Stress-strain relationship of Soil Q treated with 6.25% L after 3 drying-wetting
<i>cycles</i>
Figure 4.17 Stress-strain relationship of Soil Q treated with 6.25% L-18.75% BA after 7
drying-wetting cycles
Figure 4.18 Stress-strain relationship of Soil Q treated with 6.25% L after 7 drying-wetting
<i>cycles</i>
Figure 4.19 Stress-strain relationship of Soil Q treated with 6.25%L-18.75% BA after 9
drying-wetting cycles
Figure 4.20 Stress-strain relationship of Soil Q treated with 6.25% L after 9 drying-wetting
<i>cycles</i>

Figure 4.21 Stress-strain relationship of Soil Q treated with 6.25% L-18.75% BA after 12
drying-wetting cycles
Figure 4.22 Stress-strain relationship of Soil Q treated with 6.25% L after 12 drying-wetting
cycles
Figure 4.23 Effect of drying-wetting cycles on residual strength index (R) of Soil Q treated
with different admixture contents
Figure 4.24 Soil C samples start collapsing when submerged in water
Figure 4.25 Effect of wetting-drying cycles on residual strength index (R) of Soil C treated
with different admixture contents
Figure 4.26 Effect of BA on the relationship between penetration and load of Soil Q at 7 days
of curing
Figure 4.27 Effect of BA on the relationship between penetration and load of Soil Q at 28
days of curing
Figure 4.28 Effect of BA on the relationship between penetration and load of Soil Q at 7 days
soaked in water after 28 days of curing
Figure 4.29 Effect of lime on the relationship between penetration and load of Soil Q at 7
days of curing
Figure 4.30 Effect of lime on the relationship between penetration and load of Soil Q at 28
days of curing
Figure 4.31 Effect of lime on the relationship between penetration and load of Soil Q at 7
days soaked in water after 28 days of curing 110
Figure 4.32 Effect of L-BA on the load and penetration relationship for soil Q at 7
days of curing
Figure 4.33 Effect of L- BA on the load and penetration relationship for Soil Q at 28
days of curing
Figure 4.34 Effect of L- BA on the relationship of penetration and load for Soil Q at 7 days
soaked in water after 28 days of curing 112
Figure 4.35 CBR values of Soil Q treated with L-BA and L only at different conditions. 113
Figure 4.36 Load-penetration curves corresponding to control Soil C 115
Figure 4.37 Load-penetration curves corresponding to different conditions of Soil
C- 4.5% L

Figure 4.38 Load-penetration curves corresponding to unsoaked and soaked conditions of
Soil C-4.5% L treated with 9% and 13.5% BA at 28 days of curing
Figure 4.39 Load-penetration curves corresponding to different conditions of Soil
C-6.25% L
Figure 4.40 Load-penetration curves corresponding to unsoaked and soaked condition of
Soil C-6.25% L treated with 12.5% BA at 7 and 28 days of curing 117
Figure 4.41 Load-penetration curves corresponding to unsoaked and soaked conditions of
Soil C-6.25% L treated with 18.75% BA at 7 and 28 days curing 118
Figure 4.42 Time versus free swelling of Soil Q with different percentages of BA at
3 days curing
Figure 4.43 Time versus free swelling of Soil Q with different percentages of BA at 7 days of
curing
Figure 4.44 Effect of curing time on swelling of Soil Q treated with BA at 3 and 7 days of
curing
Figure 4.45 Time versus free swelling of Soil Q treated with different percentages of lime at
7 days of curing
Figure 4.46 Time versus free swelling of Soil Q treated with different percentages of L-BA at
7 days of curing
Figure 4.47 Free swelling value of Soil Q treated with different L-BA and L contents at 7
days of curing
Figure 4.48 Time versus free swelling of control Soil C 123
Figure 4.49 Time versus free swelling of Soil C treated with different percentages of L-BA at
7 days of curing
Figure 4.50 Effect of different contents of bagasse ash, lime and combination of lime and
bagasse ash on swell pressure of Soil Q
Figure 4.51 Void ratio versus stress of control Soil Q
Figure 4.52 Displacement versus log time of control Soil Q with stress ranging from 240 to
480 kPa
Figure 4.53 Displacement versus log time of control Soil Q with stress ranging from 640 to
800 kPa
Figure 4.54 Void ratio versus stress of Soil Q treated with 6% BA

Figure 4.55 Void ratio versus stress of Soil Q treated with 10% BA
Figure 4.56 Void ratio versus stress of Soil Q treated with 18% BA
Figure 4.57 Void ratio versus stress of Soil Q treated with 25% BA
Figure 4.58 Void ratio versus stress of Soil Q treated with different BA content
Figure 4.59 Void ratio versus stress of Soil Q treated with 1.5% L
Figure 4.60 Void ratio versus stress of Soil Q treated with 2.5% L
Figure 4.61 Void ratio versus stress of Soil Q treated with 4.5% L
Figure 4.62 Void ratio versus stress of Soil Q treated with 6.25% L
Figure 4.63 Void ratio versus stress of Soil Q treated with different L content
Figure 4.64 Void ratio versus stress of Soil Q treated with 2.5% L-7.5% BA 134
Figure 4.65 Void ratio versus stress of Soil Q treated with 4.5% L-13.5% BA 135
Figure 4.66 Void ratio versus stress of Soil Q treated with 6.25% L-18.75% BA 135
Figure 4.67 Void ratio versus stress of Soil Q treated with different L-BA content
Figure 4.68 Pre-consolidation pressure versus lime content of Soil Q treated with 6.25% L
18.75% BA and 6.25% L
Figure 4.69 Coefficient of consolidation of control and treated Soil Q with 6.25% L and
6.25% L-18.75% BA
Figure 4.70 Void ratio versus stress of control Soil C
Figure 4.71 Calculation of C_v of the control Soil C at different stresses changing from 225
to 450 kPa, using (a) log of time and (b) square root of time
Figure 4.72 Calculation of C_v of the control Soil C at different stresses changing from 450
to 900 kPa, using (a) log of time and (b) square root of time
Figure 4.73 Calculation of C_v of control Soil C at stress change from 900 to 1810 kPa using
(a) log of time and (b) square root of time
Figure 4.74 Void ratio versus stress of Soil C treated with 4.5% L
Figure 4.75 Void ratio versus stress of Soil C treated with 6.25% L
Figure 4.76 Void ratio versus stress of Soil C, treated with 6.25% L-12.5% BA 144
Figure 4.77 Void ratio versus stress of Soil C treated with 6.25% L-18.75% BA 144
Figure 4.78 Void ratio versus stress of Soil C treated with different L and L-BA contents
Figure 5.1 Schematic of direct shear cell sample (Knappetta and Craig 2012)

gure 5.2 S	Simple shear deformation of Type I and Type II soils (Budhu 2009)
gure 5.3	The effect of different additive contents and curing times on a) cohesion, 153
gure 5.4	The effect of fly ash (FA) contents and curing times on a) cohesive strength, 154
gure 5.5 l	Influence of different additive contents at 28 days curing on the peak of a) internal
ction ang	le, and b) cohesion (Gay and Schad 2000)
gure 5.6	Variation of internal friction angle with a) gray soil, and b) red soil (Harichane
al. 2011).	
gure 5.7	Variation of cohesion with a) gray soil, and b) red soil (Harichane et al. 2011).
gure 5.8 2	A specially designed mould to prepare the soil specimens in this study 159
gure 5.9	Shear stress versus horizontal displacement of Soil Q: (a) untreated soil, (b)
ated with	a 6.25% L, and (c) treated with 25% L-BA at different vertical stress
gure 5.10	Peak shear strength versus normal stress of untreated
gure 5.11	The shape of Soil Q treated with 6.25% L after completion of the test 164
gure 5.12	Variation of brittleness index of untreated and treated Soil Q
gure 5.13	Variations of shear stresses and volumetric strains of untreated and treated Soil
with sh	ear strain under effective normal stress (a) 25 kPa, (b) 50 kPa, and
100 kPa	
gure 5.1 Soil Q	4 Effect of additives on secant modulus at 50% shear strength (E50)
gure 5.15	5 Shear stress versus horizontal displacement of Soil C: (a) untreated soil, (b)
eated with	a 6.25% L, and (c) treated with 25% L-BA at different vertical stresses 170
gure 5.16	Peak and residual shear strength versus normal stress for (a) untreated soil, (b)
ated with	a 6.25% L and (c) treated with 25% L-BA 171
gure 5.17	' Failure patterns of untreated and treated specimens made of Soil C 172
gure 5.18	Variations of shear stresses and volumetric strains of Soil C versus shear strain
· (a) untre	eated, (b) treated with 6.25% L, and (c) treated with 25% L-BA 174
gure 5.19	Effect of additives on secant modulus at 50% shear strength (E_{50}) of Soil C. 175
gure 6.1	Illustration of zoning on soil water characteristic curve (Fredlund, 2006) 180
gure 6.2	Calibration suction-water content curves for wetting of filter paper
STM D- 5	5298)

Figure 6	Soil-water characteristic curves of studied colluvium soil	5
(Feuerharme	<i>l et al., 2006)</i>	l
Figure 6.4 S	VCC-w of over consolidated soils with $Pc' < sae$, for the case of $pnet = 0$ and	ł
Sres= 0, w	th possible miscalculated AEVs: (a) $\lambda\lambda p$ different from Gswae; (b) $\lambda\lambda p$	ź
Gswae (Pas	na et al, 2015)	5
Figure 6.5 T	ne procedure to measure soil matric suction using filter paper technique 188	3
Figure 6.6 S	milarity and differences among SWCC models of untreated Soil Q)
Figure 6.7 S	oil water characteristic curve of untreated Soil Q)
Figure 6.8: S	oil water characteristic curve of treated Soil Q with 6.25% L	!
Figure 6.9 S	with our characteristic curve of treated Soil Q with 6.25% L-18.78% BA 19.	!
Figure 6.10	Fitting soil water characteristic curves of untreated and treated Soil Q 192	?
Figure 6.1	The degree of saturation versus soil suction of untreated and	ł
treated Soil	2	3
Figure 6.12	Soil water characteristic curves of untreated Soil C	1
Figure 6.13	Soil water characteristic curves of Soil C treated with 6.25% L	5
Figure 6.14	Soil water characteristic curves of Soil C treated with 6.25% L-18.75% BA.19.	5
Figure 6.15	Fitting soil water characteristic curves of untreated and treated Soil C 196	5
Figure 6.16	The degree of saturation versus soil suction of untreated and treated Soil C	•
		7
Figure 7.1 T	wpical geometry of road embankment with Soil Q treated with L or L-BA (The	9
treated thick	ness of Soil Q layer thickness is ranging from 0.5 m to 2 m) 20.	l
Figure 7.2 G	eometry model of a 1m thickness layer of treated Soil Q treated with lime. 204	1
Figure 7.3	Finite element mesh with nodes of a 1m thickness layer of Soil Q)
treated with	'ime	5
Figure 7.4 C	alculation steps using PLAXIS program	5
Figure 7.5 H	inite element mesh with stress points for the geometry model of a typical In	ı
thickness lay	er of Soil Q treated with lime (a) after traffic load step, and (b) after pore wate	r
pressure (PV	$P(P) = 1 \ kPa. \dots 202$	7
Figure 7.6 L	eformation mesh of 1m thickness layer of Soil Q treated with lime	3
Figure 7.7 V	ertical settlement of 1m thickness layer of Soil Q treated with lime	3

Figure 7.8 Vertical displacements of untreated and lime-treated soil Q with different
thicknesses
Figure 7.9 Total incremental displacements model illustrating the critical failure surface,
shown by the red area
Figure 7.10 Slope stability of fill embankment under untreated and treated Soil Q with
different thickness layer of lime
Figure 7.11 Slope stability of fill embankment under untreated and treated Soil Q with
different thickness layer of lime, the date from SLOPE/W software
Figure 7.12 Apply volume strain with untreated Soil Q above water table
Figure 7.13 Effect of volumetric strain of untreated Soil Q (while only a layer with a thickness
of 1m of Soil Q treated with lime)
Figure 7.14 Vertical incremental displacement after volumetric strain applied
Figure 7.15 Vertical displacement of untreated and treated Soil Q with different thickness
layer of lime treatment after adding the volumetric strain parameter
Figure 7.16 Slope stability of fill embankment under untreated and treated Soil Q with
different thickness layer of lime after adding the volumetric strain parameter
Figure 7.17 Comparison between the Soil Q treated with 6.25% L and Soil Q treated with
6.25% L-18.75% BA a) with 0.5 and 1 m thickness layer b) with 1.5 and 2 m thickness layer.
Figure 7.18 Vertical displacement results of 4 m height fill embankment with different
thicknesses of soil treatment with lime and bagasse ash
Figure 7.19 Slope stability results for 4 m height fill embankment with a slope of 1: 2 for
different thicknesses of Soil Q treated with L-BA
Figure 7.20 Vertical displacement of Soil Q treated with 1-m thickness layer using lime –
bagasse ash with and without inclusion of geogrid reinforcement
Figure 7.21 Slope stability results of 1 thickness layer of Soil Q treated with lime – bagasse
ash with and without adding geogrid reinforcement

List of Tables

Table 2.1 Typical soil properties versus expansion potential (Day 2010). 1	1
Table 2.2 Clay activity based on clay minerals (Skempton, 1953).	9
Table 2.3 Shrinkage potential of soils bested on plasticity index (PI) and clay fraction (BRI	E,
1993)	9
Table 2.4 Classification of expansion potential using free swell ratio (FSR) (Prakash an	d
Sridharan, 2004)	2
Table 2.5 Classification of expansive soils based on the standard absorption moisture conter	nt
(SAMC), plasticity index (PI) and free swell values (CMC, 2004)	2
Table 2.6 Ranges of COLE to govern soil expansion potential (USDA-NRCR, 2005) 2	3
Table 2.7 Expansive soil treatment alternatives (Nelson and Miller, 1992).	5
Table 2.8 Chemical composition of bagasse ash determined from different studies	!4
Table 2.9 Studies utilized bagasse ash (BA) to improve geotechnical properties of different	et
soils	!8
Table 3.1 Notation of materials used in the experimental program. 5	7
Table 3.2 Chemical component of hydrated lime (Adelaide Brighton Cement Ltd)	8
Table 3.3 Summary of mixed soils, lime (L) and bagasse ash (BA)	60
Table 3.4 Specific Gravity of materials	52
Table 3.5 Effect of lime (L) and bagasse ash (BA) on Soil C after 1 day of curing	52
Table 3.6 Effect of bagasse as (BA) and lime (L) on the dry density of Queensland soil of	at
36.5% water content	7
Table 3.7 Effect of bagasse as (BA) and lime (L) on the dry density of Soil C at 30% water	?r
content	8
Table 3.8 Summary of unconfined compressive of mixing soil with various admixture conten	ts
at different curing time	'1
Table 3.9 Summary of durability test for soils treated with different admixture content 7	'3
Table 3.10 CBR test outline of soils at unsoaked and soaked condition after 7 and 28 days of	of
curing with different lime and bagasse ash contents7	'6
Table 3.11 Free swelling test of soils treated with various additive content	'7
Table 3.12 Summery of swell pressure and consolidation tests	'9

Table 3.13 Properties of expansive soils. 81
Table 4.1 Linear shrinkage of Soil Q mixed with various additives at 7 and 28 days of curing.
Table 4.2 Summary of linear shrinkage results of Soil C treated with special admixture
contents at different curing days
Table 4.3 Free swell ratio (FSR) and expansivity of control and treated Soil Q with different
contents of lime-bagasse ash
Table 4.4 UCS data of Soil Q treated with different admixture contents after several drying-
wetting cycles
Table 4.5 Data of average UCS and residual strength index (R) of Soil C treated with
different admixture contents after several wetting-drying cycles
Table 4.6 CBR values of the control soil and Soil Q treated with different percentages of
bagasse ash
Table 4.7 CBR values of Soil Q treated with different percentages of lime
Table 4.8 CBR values of Soil Q treated with different percentages of L-BA
Table 4.9 Summary of CBR results of control and treated Soil C at different conditions. 118
Table 4.10 Summary of compression and swell indices of Soil Q treated with BA and L-BA.
Table 4.11 Summary of compression and swell indices of Soil Q treated with L and L-BA.
Table 4.12 Summary of swell pressure (Ps), compression index (Cc), swell index (Cs), pre-
consolidation pressure ($\sigma c'$) and coefficient of consolidation (Cv) of Soil C treated with
different admixture content
Table 5.1 Shear strength parameters of soft clay with different cement contents and 152
Table 5.2 Direct shear tests for Halawa soil (Ding 2000)
Table 5.3 Comparison between peak and residual shear strength parameters of Soil 164
Table 5.4 Comparison between peak and residual shear strength parameters of Soil C. 172
Table 6.1 Fredlund and Xing's model parameters of untreated and treated Soil Q taken from
the SWCC (the gravimetric water content versus soil suction in logarithmic scale) 190
Table 6.2 Fredlund and Xing's model parameters of untreated and treated Soil Q taken from
SWCC (the degree of saturation versus soil suction in logarithmic scale)

Table 6.3 Fredlund and Xing's model parameters of untreated and treated Soil C taken	from
SWCC based on the gravimetric water content versus soil suction	196
Table 6.4 Fredlund and Xing's model parameters of untreated and treated Soil C taken	from
SWCC based on the degree of saturation versus soil suction	196
Table 7.1 Summary of pavement material properties (Khan and Abbas, 2014)	202
Table 7.2 Summary of finite element model parameters for materials used	203
Table 7.3 Safety factor of embankment with different thickness layer of Soil Q treated	with
lime	211