MODELLING THE STRUCTURAL BEHAVIOUR OF THE FOLD-AWAY SHELTER

A thesis submitted for the degree of Master in Engineering (Research) University of Technology, Sydney

> Thomas Rolan E. Rondero March 2006

CERTIFICATE OF AUTHORSHIP/ORIGINALITY

I certify that the work in this thesis has not previously been submitted for a degree nor has it been submitted as part of requirements for a degree except as fully acknowledged within the text.

I also certify that the thesis has been written by me. Any help that I have received in my research work and the preparation of the thesis itself has been acknowledged. In addition, I certify that all information sources and literature used are indicated in the thesis.

Signature of Candidate

Production Note: Signature removed prior to publication.

ACKNOWLEDGEMENTS

The research reported in this thesis was made possible through an Australian Academic International Development (AusAID) grant from 2000 to 2003.

I am grateful to Dr Florentino O. Tesoro and Dr Florence P. Soriano, past and present Directors of the Forest Products Research and Development Institute (FPRDI), Department of Science and Technology, for allowing me to undertake this study on study leave.

Dr Keith Crews, Associate Professor and Deputy Director of the Centre for Built Infrastructure Research of the University of Technology, Sydney (UTS), supervised this research. Dr Crews' diligence, enthusiasm for research and trust served as encouragement throughout the project.

Dr Ali Saleh, Senior Lecturer of the Infrastructure and the Environment Group of the Faculty of Engineering of UTS, co-supervised this research. Dr Saleh's patience and diligence in personally teaching me finite element modelling was of great help throughout the project.

Professor Bijan Samali, Director of the UTS Centre for Built Infrastructure Research and Head of the Infrastructure and the Environment Group of the Faculty of Engineering of UTS, provided an invaluable contribution to the completion of the project. I am very grateful for his assistance.

Mr Mario Benitez, Manager of the Structures Laboratory, advised and assisted me in all my experiments. I am very grateful.

To the staff of the Structures Laboratory, namely, Messrs Laurence Stonard for being always available for my instrumentation requirements, David Hooper for providing me with the materials needed for the experiments, and Warwick Howse and Wolfgang Stengl for assisting me with my experimental set-up and testing, I am very grateful.

I thank my wife, Rose, for being always there; for her prayers, love and patience.

I also thank my son, Theodore Ransel, for believing in me.

Finally, praises and thanks to Jesus Christ, the Servant King, to whom all honour and glory are due.

ABSTRACT

The F-shelter underwent non-destructive monotonic load tests and destructive shake table test. The timber-framed shear walls with different sheathings; namely wood wool cement boards (wwcb) and F11 structural plywood were tested under uniaxial loading. Furthermore, finite element models (FEM) supplemented the experimental work. An FEM of the corner metal bracket and a 2-dimensional FEM for the timber-framed shear wall were generated and verified from the experimental work.

Behavioural responses from unidirectional lateral loading of the wall were obtained. For the dynamic test, the Kobe earthquake and Zone IV earthquake were simulated to determine the dynamic response of the F-shelter. Excitation was limited to 70% full scale displacement record of Kobe and 80% of Zone IV, due to the 100mm limitation in the allowable displacement on both sides of the shaker table. The shake table test showed that the F-shelter can withstand the simulated earthquakes.

FEMs were developed using ANSYS 7.2, a general finite element software. A requisite input data for the timber-framed shear wall FEM in lieu of a hinge connection corner joint for the timber-framed shear wall were generated through experimental work on the corner metal brackets and verified with the generated FEM. The results of the FEM of the *Dipterocarpus grandiflorus Blanco* (Apitong) timber-framed sheathed with wwcb were 5% to 9% higher than the average values for maximum deflections and maximum load capacity. The FEM results of the Radiata pine sheathed with F11 plywood, however, were 25% to 14% lower than the average values for maximum deflections and maximum load capacity.

This thesis has demonstrated the process of generating FEMs that can be used as a tool to improve and modify the F-shelter. The structural reliability of design and construction of the first F-shelter prototype was verified from the whole house test and structural modeling of the wall using FEM.

A	cknowledgeme	enti	
A	Abstractii		
Li	ist of Tables		
Li	ist of Figures.	х	
Li	ist of Appendic	xesxiv	
1	Introductio	n1	
	1.1 Objective	1	
	1.2 Scope of v	works2	
2	2 Background	1	
	2.1 Rationale		
	2.2 General d	escription of the F-shelter technology5	
	2.3 Design cri	teria10	
	2.4 Importance	e of the finite element modelling10	
3	3 Literature r	eview13	
	3.1 Introduction	on13	
	3.2 The Philip	ppines, a calamity-prone area13	
	3.2.1 Eart	hquakes	
	3.2.1.1	Tectonic earthquakes13	
	3.2.1.2	Volcanic earthquakes14	
	3.2.2 Tro	pical typhoons14	
	3.3 Experime	ntal works17	
	3.3.1 Intro	oduction	
	3.3.2 Met	al brackets	
	3.3.3 She	ar wall testing20	
	3.3.4 Hou	se testing	
	3.4 Finite ele	ment models	
	3.4.1 Intro	oduction23	
	3.4.2 She	ar wall modelling23	
	3.5 Summary	/	

Table of Contents

4	Materials and experimental procedures	
	4.1 Introduction	
	4.2 Materials	
	4.3 Experimental procedures	
	4.3.1 Preliminary tests	
	4.3.1.1 Fastener withdrawal test	
	4.3.1.1.1 Screw withdrawal test specimen	
	4.3.1.1.2 Nail withdrawal test specimen	
	4.3.1.1.3 Procedure	
	4.3.1.1.4 Instrumentation	
	4.3.1.2 Lateral nail and screw resistance test	
	4.3.1.2.1 Specimen	
	4.3.1.2.2 Procedure	
	4.3.1.2.3 Apparatus	
	4.3.2 Wall corner test with metal brackets	
	4.3.2.1 Test specimen	
	4.3.2.2 Procedure	
	4.3.2.2.1 Assembly	
	4.3.2.2.2 Testing	
	4.3.2.3 Instrumentation	
	4.3.3 Monotonic test on wall	
	4.3.3.1 Introduction	
	4.3.3.2 Specimen	
	4.3.3.2.1 Sheathing panels47	
	4.3.3.2.1.1 Wood wool cement boards	
	4.3.3.2.1.2 Plywood	
	4.3.3.2.2. Timber framing	
	4.3.3.2.2.1 Apitong	
	4.3.3.2.2.2 Radiata Pine	
	4.3.3.2.3 Frame-to-sheathing connector	
	4.3.3.3 Construction method	
	4.3.3.3.1 Apitong-framed wwcb	
	4.3.3.3.2 Radiata pine-tramed F11 plywood	
	4.3.3.4 Test set-up	
	4.3.3.6 Testing procedure	
	4.3.3.7 Shear wall property definitions	
	4.3.3.7.1 Load-displacement parameters	
	4.3.3.7.2 Wall capacity 55	

	4.3.3.7.3 Wall failure	
	4.3.3.7.4 Energy dissipation	
	4.3.3.7.5 Equivalent Energy Elastic Plastic (EEEP) parameters	
	4.3.3.7.5.1 Elastic stiffness	
	4.3.3.7.5.2 Yield load and yield displacement	
	4.3.3.7.5.3 Ductility	
	4.4 Summary	59
5	Experimental results	60
	5.1 Introduction	60
	5.2 Preliminary tests	60
	5.2.1 Introduction	60
	5.2.2 Withdrawal test	60
	5.2.2.1 Nail	60
	5.2.2.2 Screw	60
	5.2.3 Lateral resistance test	62
	5.2.3.1 Nail with wwcb sheathing	62
	5.2.3.2 Nail with plywood sheathing	
	5.2.4 Corner joint testing with metal brackets	
	5.2.4.1 Apitong framing	64
	5.2.4.2 Radiata pine framing	66
	5.3 Monotonic test on walls	67
	5.3.1 Introduction	67
	5.3.2 Comparison of two timber-framed walls	67
	5.3.2.1 Load-displacement relationship	67
	5.3.2.2 Ultimate load	
	5.3.3 Comparison of the walls with different sheathings	69
	5.3.3.1 Load-displacement relationship	69
	5.3.3.2 Ultimate load	71
	5.3.3.3 Yield load and displacement	72
	5.3.3.4 Failure capacity and displacement	73
	5.3.3.5 Ductility	74
	5.3.3.6 Work to failure or energy dissipation	
	5.3.3.7 Wall behaviour and mode of failure	
	5.3.3.7.1 Wall sheathed with wood wool cement boards	76
	5.3.3.7.2 Wall sheathed with F11 plywood	79
	5.4 Summary	81

6	F-shelter tests	
	6.1 Introduction	
	6.2 Full-size F-shelter testing	
	6.2.1 The F-shelter for testing	
	6.2.2 Construction and assembly	
	6.3 Monotonic test	
	6.3.1 Set-up and experimental procedure	
	6.3.2 Instrumentation	
	6.3.3 Results and conclusions	
	6.3.3.1 F-shelter with metal plate reinforcements	
	6.3.3.2 F-shelter without metal plate reinforcements	
	6.4 Dynamic test through the Shaker Table95	
	6.4.1 Ground motion simulation	
	6.4.1.1 Kobe earthquake ground simulation	
	6.4.1.2 Zone IV earthquake ground simulation	
	6.4.2 Set-up and instrumentations	
	6.4.3 Experimental procedure	
	6.4.4 Results and conclusions101	
	6.4.4.1 Acceleration comparison102	
	6.4.4.2 Visual inspection105	;
	6.5 Summary106	
7	Finite Element Modelling the non-linear behaviour of the timber-framed	
	wood wool cement board and plywood sheathing	
	7.1 Introduction	
	7.2 Modelling the corner bracket 107	
	7.3 Modelling the wall	
	7.3.1 Framing	
	7.3.2 Corner joint with metal brackets114	
	7.3.3 Sheathing 117	
	7.3.4 Fasteners	
	7.3.5 Support	
	7.3.6 Loading	
	7.4 Preliminary FE model result	
	7.5 Summary	

8	Discussions for Experimental and FEM results	125
	8.1 Introduction	125
	8.2 Results from the Finite Element Model	125
	8.2.1 Corner bracket stiffness	
	8.2.2 Non-linear behaviour of the wall	
	8.2.2.1 Wall frames	126
	8.2.2.2 Wall with sheathings	
	8.2.2.2.1 Wood wool cement board sheathing	
	8.2.2.2.2 F11 structural plywood sheathing	135
	8.3 Discussions	138
	8.3.1 Non-linear behaviour of the timber frame wall	138
	8.3.1.1 Comparison with the static tests	138
	8.3.1.1.1 Timber-framed wall only with let-in metal braces	
	8.3.1.1.2 Timber-framed wall with sheathings	139
	8.3.1.1.3 Analysis	140
	8.3.1.1.3.1 Effect of timber frame MOE to ductility	
	and load capacity	141
	8.4 Summary	142
9 (Conclusions	143
	9.1 Introduction	143
	9.2 Corner metal brackets	143
	9.2.1 Experimental	143
	9.2.2 FEM	144
	9.3 Full size testing	144
	9.3.1 Monotonic test on wall	
	9.3.2 Static test of the F-shelter	144
	9.3.3 Dynamic test of the F-shelter	144
	9.4 Alternative timber frame, sheathing material and fastener	145
	9.5 Finite element model of the wwcb and plywood sheathed wall	145
10	Recommendations and areas for future research	146
	10.1 Recommendations	146
	10.2 Areas for future research	
Re	ferences	149
Ap	pendix	153

•

List of Tables

		page
Table 2.1	Report on a flood calamity and figures on affected individuals.	3
Table 3.1	Annual Frequency of Tropical Typhoons within the Philippine Area	16
	of Responsibility (PAR) for the years 1948-2000 (NCMC, 2001)	
Table 4.1	Summary of materials and properties.	29
Table 4.2	Number of replicates for each joint type.	39
Table 4.3	Number of tests and specimen labels for various wall frames.	46
Table 4.4	Sheathing materials and nailing schedule.	47
Table 4.5	Elevation view with instrumentation locations for displacement	53
	measurement.	
Table 5.1	Nail withdrawal test results for 10 wood prisms.	61
Table 5.2	Maximum loads and corresponding displacements.	61
Table 5.3	Peak loads and displacements per fastener for wwcb using	62
	Apitong timber.	
Table 5.4	Peak loads and displacement for plywood using Radiata pine timber	63
Table 5.5	Average peak loads and displacements for Apitong framing.	66
Table 5.6	Average peak loads and displacements for Radiata pine framing.	66
Table 5.7	Monotonic peak loads on wall frames.	68
Table 5.8	Monotonic peak load values for the walls with different sheathings.	72
Table 5.9	Elastic stiffness of the walls.	73
Table 5.10	Monotonic yield load and displacement values.	73
Table 5.11	Failure load and displacement values.	74
Table 5.12	Ductility ratios of walls with different sheathing materials.	74
Table 5.13	Energy dissipation of sheathing materials.	75
Table 6.1	Summary of channels and measured displacements.	89
Table 6.2	Summary of deformation for static test.	95
Table 6.3	Load percentage applied to the structure for a particular earthquake	100
	simulation.	
Table 6.4	Visual inspection report.	105
Table 7.1	Input properties for the corner bracket model.	108
Table 7.2	Input properties for the LINK 1 element for screw.	109
Table 7.3	Input properties for BEAM 3 element.	113
Table 7.4	Equivalent input values for the non-linear LINK 1 element.	116
Table 7.5	Apitong-framed corner with metal brackets.	116
Table 7.6	Radiata pine-frmaed corner with metal brackets.	117

Table 7.7	Load and deflection input data for real constants for a COMBIN 39	122
	for lateral nail resistance test for a wwcb and plywood sheathing.	
Table 8.1	FEM vs. Experimental work for Apitong-framed corner bracket in	126
	tension.	
Table 8.2	Tabulated results of experimental versus FEM.	128
Table 8.3	Load and displacement values from the FEM analysis.	131
Table 8.4	FEM versus the experimental averaged value.	137

List of Figures

		page
Figure 1.1	Workflow for the development of the mobile foldaway F-shelter.	2
Figure 2.1	Some makeshift shelters used in the aftermath of calamities.	5
	a. Structures made of bamboo poles and roofs with clothe and	
	b. Structure is supported by tree braches, bundled leaves as walls	
	and corrugated galvanized iron sheets tied on top serving as roof.	
Figure 2.2	The emergency shelter is delivered to the site in a rigid case. The	6
-	rigid case is mounted on adjustable prefabricated footings.	
Figure 2.3a	Floor plan of the F-shelter	6
Figure 2.3b	Front elevation of the F-shelter	7
Figure 2.3c	Rear elevation of the F-shelter	7
Figure 2.3d	Left side elevation of the F-shelter	8
Figure 2.3e	Right side elevation of the F-shelter	8
Figure 2.4	Floor plan; shown here are walls and interconnectors	9
Figure 2.5	The lumber-framed shop-fabricated emergency shelter, complete	9
	with false post (used to cover wall to wall connection), stairs and	
	tie down straps anchored to the ground, currently being service	
	tested at FPRDI.	
Figure 3.1	Three wind zones in the Philippines (NSCP, 2001).	15
Figure 3.2	Frequency of Tropical Cyclone Passage (NCMC, 2001).	17
Figure 3.3	Elevation plan and joint details for a metal let-in bracket	19
	(<u>www.icc-es.org</u> , 2006).	
Figure 3.4	Finite element model of the wall (Hite and Shenton, 2002).	24
Figure 3.5	A detailed model of the wall; a. with sheathings and b. studs only	26
	(Kasal and Leichti, 1992a).	
Figure 3.6	Equivalent wall (Kasal and Leichti, 1992a).	26
Figure 4.1	General flow of the project activities.	28
Figure 4.2	a. Assembly for screw withdrawal test and b. Dimension of the	31
	and location of the screw.	
Figure 4.3	Fabricated gripping device with screw on centre.	32
Figure 4.4	Schematic diagram of the Instron testing machine.	33
Figure 4.5	Schematic diagram of the test.	34
Figure 4.6	Schematic diagram for lateral resistance test.	35
Figure 4.7	Wood wool sheathing specimen for lateral nail resistance test.	36
Figure 4.8	Plywood sheathing specimen for lateral nail resistance test.	36

Figure 4.9	Spherical seat fitted in the UTM	36
Figure 4.10	LVDT that is used to measure deflection	37
Figure 4.11	Location of the corner metal brackets in the wall frame	38
Figure 4.12	Schematic diagrams of structural grade corner metal brackets;	40
	a. 82-degree angle metal brackets, b. 90-degree angle metal	
	brackets, and c. 98-degree angle metal brackets	
Figure 4.13	Actual wall corner joint specimens with timber frame and metal	41
	brackets; a. 82-degree corner joint, b. 90-degree corner joint,	
	and c. 98-degree corner joint.	
Figure 4.14	Connection fittings used for the compression test; a. top connection	42
	and b . bottom connection.	
Figure 4.15	Connection fittings used for the tension test; a. top connection and	43
	b. bottom connection.	
Figure 4.16	Compression test set-up on the UTM for a corner joint with a metal	44
	bracket.	
Figure 4.17	Tension test set-up on the Instron for a corner joint with metal brackets	45
Figure 4.18	Typical wall framing for the wall specimen	47
Figure 4.19	Plywood sheathed wall set up for lateral resistance test.	50
Figure 4.20	Schematic diagram of the wall test set-up; a . full set-up and b .	51
	enlarged view of the specimen's bottom.	
Figure 4.21	Worm's eye view of the hydraulic jack and load cell with attached	52
	roller fitting.	
Figure 4.22	Elevation view with instrumentation locations for displacement	53
	measurement.	
Figure 4.23	Lateral resistance test set-up for the wall.	54
Figure 4.24	Equivalent Energy Elastic Plastic (EEEP) curve (Salenikovich, 2000).	56
Figure 5.1	Nail tear through the wwcb.	63
Figure 5.2	Mode of failure; a . separation from the Radiata pine and b . enlarged	64
	view of nail pulling out from the frame.	
Figure 5.3	Compression and tension test graphs for the 98-degree corner metal	65
	bracket.	
Figure 5.4	Joint separation and buckling of the brace.	65
Figure 5.5	Typical load-deflection curve on tested Apitong and Radiata pine	68
	timber frame.	
Figure 5.6a	Load-deflection curve for wwcb and plywood-sheathed timber frames.	69
Figure 5.6b	Averaged load-deflection curves for wwcb and plywood sheathing.	70
Figure 5.7	A gap of 5mm on the far end wall.	71
Figure 5.8	Tearing of the nails through the wwcb panels.	77

Figure 5.9	Pulling out of the nails from the timber framing.	77
Figure 5.10	Separation of the vertical frame from the base of the wall.	78
Figure 5.11	End separation with the other sheathing removed.	78
Figure 5.12	Nail pull-out from the timber framing.	79
Figure 5.13	Nail pull-out starts from the base where top load is applied.	80
Figure 5.14	Separation on the nail end grain connection for the vertical bottom plate.	80
Figure 6.1	Base of the structure for testing.	83
Figure 6.2	Floor fastened to the steel base frame.	84
Figure 6.3	Side walls 2 and 6 were erected.	85
Figure 6.4	Location of two hydraulic jacks.	86
Figure 6.5	Reinforcement plates used to connect the roof and wall.	87
Figure 6.6	Hydraulic rams connected to the reaction frame.	88
Figure 6.7	Location of instrumentation channels for non-destructive testing.	89
Figure 6.8	Flow of testing activity for the F-shelter.	90
Figure 6.9	Hysteresis loops at 4kN maximum load on the side with opening.	92
Figure 6.10	Hysteresis loops at 4kN maximum load on the side without opening.	93
Figure 6.11	Deformation on the side without window opening.	94
Figure 6.12	Deformation on the side with window opening.	94
Figure 6.13	Time history record for Kobe earthquake.	97
Figure 6.14	Time history record for Zone IV earthquake.	98
Figure 6.15	Locations of PCB 393C accelerometers and LVDTs.	99
Figure 6.16	Locations of the video camera recorders.	99
Figure 6.17	Schematic diagrams of activities for the dynamic test of F-shelter.	101
Figure 6.18	Acceleration @ 50% Kobe on the loaded side; with metal plates.	103
Figure 6.19	Acceleration @ 50% Kobe on the opposite side; with metal plates.	103
Figure 6.20	Acceleration @ 50% Kobe on the loaded side; without metal plates.	104
Figure 6.21	Acceleration @ 50% Kobe on the opposite side; without metal plates.	104
Figure 7.1	Schematic diagram of FEM for the corner bracket.	108
Figure 7.2	Load and deflection curves of the screw withdrawal tests.	109
Figure 7.3	Schematic diagram of the wall model.	110
Figure 7.4	Geometry of the timber frame.	111
Figure 7.5	BEAM 3; 2D Elastic Beam (ANSYS 1998).	112
Figure 7.6	End connection for coincident nodes.	112
Figure 7.7	BEAM 3 elements for the timber framing.	113
Figure 7.8	Effective length to be used in the 90-degree corner joint model.	114
Figure 7.9	Load-deflection curves for FEM and average results from the	115
	82-degree corner made with Radiata pine frame.	
Figure 7.10	Blow-up sketch of wwcb from the sheathing.	118

Figure 7.11	PLANE 42; 2D Structural Solid (ANSYS 1998).	119
Figure 7.12	PLANE 42 element to model the wall sheathing.	119
Figure 7.13	COMBIN 39 Nonlinear spring (ANSYS 1998).	120
Figure 7.14	Deformations of two COMBIN 39 element.	120
Figure 7.15	Load-deflection curve for lateral nail resistance test for plywood	121
	sheathing.	
Figure 7.16	Set-up of supports.	122
Figure 7.17	Location of the load on the wall specimen.	123
Figure 7.18	Typical load-deflection curve from FE model and average from wall	124
	tests of Apitong frame with wwcb sheathing.	
Figure 8.1	Load-deflection curve for the two wood-framed walls.	127
Figure 8.2	FEM and average load-deflection curves for the Apitong-framed wall.	127
Figure 8.3	FEM and average load-deflection curves for Radiata pine-framed wall.	128
Figure 8.4	Nodal solution for x-direction displacement of Apitong wall frame	129
	at P <i>ma</i> x.	
Figure 8.5	Deformed and undeformed shape of the Radiata pine wall frame	130
	at P <i>max.</i>	
Figure 8.6	Nodal solution image at 2.28mm displacement for stress (x-direction	132
	In the sheathing panels).	
Figure 8.7	Nodal solution image at 4.56mm displacement for stress (x-direction	132
	In the sheathing panels).	
Figure 8.8	Nodal solution image at 7.02mm displacement for stress.	133
Figure 8.9	Nodal solution image at 10.6mm displacement for stress.	133
Figure 8.10	Nodal solution image at 19mm displacement for stress.	134
Figure 8.11	Load-deflection curves for wwcb-sheathed wall Apitong frame.	135
Figure 8.12	Plywood-sheathed wall model.	136
Figure 8.13	Load-deflection curve for the plywood-sheathed Radiata pine timber	137
	frame.	
Figure 8.14	FEM, EEEP line and loads corresponding to NSCP edition.	140
Figure 8.15	Effect of MOE of timber frame to ductility based on FEM analysis.	141
Figure 8.16	MOE vs. Pmax based on FEM analysis without corner metal brackets.	142

List of Appendices Located on compact disk

Appendix A	F- shelter concept page 153
	1. Steps in erecting the Foldaway-shelter
Appendix B	Philippine archipelago with its bounding trenches & subduction zones and active faults.
Appendix C	Seismicity zone map of the Philippines
Appendix D	Seismic zone map of the Philippines
Appendix E	Distribution of earthquake generators in the Philippines
Appendix F	Distribution of active and inactive volcanoes of the Philippines
Appendix G Wall corner joint test	
	 Raw test results for Apitong and Radiata Pine frame in data file Test results in Excel file 82-degree Radiata Pine in tension Summary Average versus FEM
Appendix H	Lateral nail resistance test
	 Plywood sheathing in Excel file Wood wool cement board sheathing in Excel file
Appendix I	Apitong wall frame test data
	 Test results in Excel file Summary of the test results Average versus experiment test results Average versus FEM
Appendix J	Radiata pine wall frame test data
	 Test results in Excel file Summary of test results Load-deflection curves Average versus FEM
Appendix K	Wall test with wwcb sheathing
	 Test results in Excel file Summary of test results Awcb and plywood load-deflection curves Average load-deflection curves for wwcb and plywood Load-deflection curves for wwcb 1–3 Load-deflection curves for wwcb1, wwcb2 and average Wwcb versus FEM

Appendix L	Wall test with plywood sheathing
	 Test results in Excel file Summary of test results a. Wwcb and Plywood load-deflection curves b. P1 versus FEM
Appendix M	F-shelter test
	 quasi-static test without metal plates a. 2, 3, 4 and 5kN test results in Excel file b. Summary for 5kN with and without metal plates quasi-static test with metal plates a. 1.5, 3 and 4kN test results in Excel file b. LC1-LVDT11 c. LC2-LVDT3 d. side with opening
	 e. side without opening 3. dynamic test without metal plates a. 20, 50 70% of full excitation of Kobe earthquake I. Acceleration at top corner on loaded side; 50% Kobe (without reinforcements) II. Acceleration at top corner on opposite side; 50% Kobe (without reinforcements) b. 50% of full excitation of Zone IV
	 4. dynamic test with metal plates a. 5, 15, 30, 40 and 50% of full excitation of Kobe earthquake I. Acceleration at top corner on loaded side; 50% Kobe (with reinforcements) II. Acceleration at top corner on opposite side; 50% Kobe (with reinforcements) b. 50 and 80% of full excitation of Zone IV
Appendix N	ANSYS files
	 Apitong wall frame Deformed shape at Pmax Nodel solutions
	 Radiata Pine wall frame a. Deformed shape at Pmax b.* Nodel solutions
	 3. Wwcb sheathed wall a. Deformed shape at Pmax b. Nodal stresses at x and y directions c. Nodal solutions 4. Plywood sheathed wall
	 a. Deformed shape at Pmax b. Nodal stresses at x and y directions c. Nodal solutions