



**UNIVERSITY OF
TECHNOLOGY SYDNEY**

**Determination of Embedded Length and General
Condition of Utility Poles Using Non-Destructive
Testing Methods**

By
Amir Zad

A thesis submitted in fulfilment
of the requirements for the degree of
Doctor of Philosophy

Faculty of Engineering and Information Technology
University of Technology Sydney

October 2013

CERTIFICATE OF AUTHORSHIP/ORIGINALITY

I certify that the work in this thesis has not previously been submitted for a degree nor has it been submitted as part of requirements for a degree except as fully acknowledged within the text.

I also certify that the thesis has been written by me. Any help that I have received in my research work and the preparation of the thesis itself has been acknowledged. In addition, I certify that all information sources and literature used are indicated in the thesis.

Signature of candidate

Amir Zad

October 2013

Abstract

Timber utility poles play a key role for electricity distribution systems in Australia and in many other countries. There are over 5 million timber utility poles currently used in Australian energy networks which are more than 80% of total utility poles in the network. Lack of knowledge about the current condition of existing poles such as embedded length, the degree of deterioration and damage below the ground level or on top of the pole, leads to uncertainty for replacement or maintenance works. Hence, it is essential to develop a cost effective and reliable non-destructive method to ensure safety and to reduce maintenance costs.

Different Non-destructive Testing (NDT) methods such as Sonic Echo, Bending Waves and Ultraseismic methods have been used in field applications over the past decades as simple and cost-effective tools for identifying the condition and underground depth of embedded structures, such as timber poles or piles in service. Despite the wide spread use of these methods by consultants around the world, reports describing field applications have shown that the results lack both consistency and reliability. Difficulties faced in field applications are often associated with complicated and imperfect/deteriorated materials, environmental effects, interaction of soil and structure and unknown boundary conditions, which lead to a great deal of uncertainties. In order to address this problem and develop reliable methods for embedment length determination and identification of damage below ground level, an R&D program commenced in 2008 at the University of Technology Sydney in collaboration with the Electricity Network Association of Australia. The aim of this study is to investigate and future develop the current non-destructive test methods with acceptable accuracy and repeatability, whilst being cost efficient for condition assessment of timber poles and piles as a part of the main program.

To tackle the problems and evaluate effects of various factors associated with timber materials, on these NDT methods, thorough numerical investigations using Finite Element (FE) was necessary. In this study on isotropic model was used for timber material as the main object of the numerical study was to get a better understanding of wave travel in materials without any other uncertainties. The numerical evaluation will start with a free timber pole without embedment to understand the behaviour of the

timber poles under surface NDT Methods. The results will be used for benchmarking in further investigations involving structure and soil interaction and boundary conditions. The model is verified with static analysis. Then, the FE beam model is enhanced with more advanced features requiring more steps to simulate other boundary conditions. According to the results, stress wave velocity will decrease with increase in embedded length. Therefore, two different velocities, one for stress wave travelling above the soil level and one travelling inside the soil with around 20% decrease in velocity was calculated. The error of length estimation averaged between 5% and 9% depending on the boundary conditions and the reference sensor for calculations.

In order to address this problem and develop a reliable method for embedment length and identification of damage below ground level, also the bending wave method is fully investigated and verified for the potentials and limitations. The success of determining these parameters (embedded length and location of damage) mainly depends on the accuracy of measuring the bending wave velocity. However, bending wave is highly dispersive in nature and, hence, it is important to find its frequency dependent velocity. Short Kernel Method (SKM) has been used as a signal processing tool to calculate the frequency dependent velocity and also the embedded length. As there are no guidelines to select those kernel frequencies, different kernel frequencies were selected based on the results of FFT and then applying the SKM method. As a result of the bending wave velocity investigation, the appropriate kernel frequency is identified to be between 600 to 800 Hz. The results are verified using Bernoulli-Euler Beam theory and Timoshenko beam theory. Based on the length estimation, the kernel frequency of between 650 Hz to 800 Hz will result in less than 8% error in embedded length estimation.

Furthermore, the Ultraseismic method is also applied on the results of timber modelling. Based on the results of velocities below and above the soil, the stress wave velocity is decreased by 22% overall below the soil in comparison with stress wave velocity above the soil. Based on the Ultraseismic method, the length of the timber pole is estimated by cross correlating the first arrival and reflection waves. Ultraseismic test applying impact at the middle was also investigated for a 12m timber pole. It was found that, impact at middle of the specimen generated two compressional waves (travelling down and reflecting at the butt) and tensile waves (travelling up and reflecting at the top). This wave interference makes the analysis complicated. In addition, impact from the middle

with 45 degree angle generates the combination of horizontal and vertical forces which result in contribution of bending waves to longitudinal waves. As a result, the signal will include multiple wave modes which are required to be separated before calculation of velocity and length determination. It should be mentioned that, Timber pole is modelled as an isotropic material here and if the anisotropy of the material is included the analysis will be more complicated.

This study also presents the results of Sonic Echo, Impulse Response, Bending Wave and Ultraseismic methods, investigated for determining the stress wave velocity and embedded length of poles with different testing conditions in the structural laboratories at UTS and in the field at Mason Park (NSW) and Horsham (Victoria).

According to the laboratory results, the coefficient of variation of velocity estimation of timber pole is relatively higher than steel beam and timber beam due to uncertainties in timber material such as anisotropy of timber material, stress direction in regards to grain angle of timber, location of a sensor relative to other sensors in regards to the annual growth ring orientation and existence of knots or any imperfections in timber. Choosing the reflection peak for length determination is one of the main parts of these methods and this could be affected by geotechnical conditions. Based on the results, the stress wave velocity will decrease inside the soil and a reduction factor is required to be applied to stress wave velocity above the soil to obtain the stress wave velocity below the soil. This reduction factor varies depending on the different testing/boundary conditions as well as the soil depth. In SE method, the scatter of the average error for the pole specimen, associated with different tests, ranged between 1% and 20% for all cases except layer 6 with 26% using sensor 1 for calculations. By using sensor 2 for estimation of the length, the average error becomes less than 9% for all cases except for layer 7 with 32%. However, more uncertainties are involved in terms of length calculation using sensors 3 and 4 located 1.5m and 2m from the impact location in comparison with using sensors 1 and 2 for calculations.

The phase velocity is calculated for each kernel frequency under different pull out testing conditions. Also based on the results of bending wave method, the kernel frequency between of 400-800 Hz was identified for use in SKM method for phase velocity calculations. Using the SKM to estimate the length of the pole with Bending Wave method for a 5m timber pole under different pull out conditions shows the

percentage of error for all boundary conditions to be between -10.5% and 0%. If the kernel frequency above 600Hz is selected, the average error for length estimation becomes less than 5% for most boundary conditions.

Also Ultraseismic method was considered for stress wave velocity estimation of timber poles impacted at the top. According to the results, using sensors close to impact location (up to 2-3m) will result in good estimation of the velocity calculations. However, these will not necessarily lead to accurate estimations. According to the results, the average error in length determination for timber poles under different pull out conditions which is more relevant to timber poles in-service is less than 18%. According to the results for Ultraseismic method using impact at the top in Horsham, the stress wave velocities were calculated with relatively good accuracy.

By considering relatively good and damaged poles in Horsham, it was found that the severe termite damage can be identified by the irregular patterns of FFT from impacted timber pole. This can be used to classify which timber poles are required to be replaced in the field.

Acknowledgements

This PhD project could not have been possible without the support provided by numerous people. In particular, the author would like to express his appreciation and gratitude to his principal supervisor, Associate Professor Jianchun Li for his outstanding guidance, encouragement, wisdom and caring support provided throughout this project. Utmost gratitude is also forwarded to Prof. Bijan Samali who had given the author invaluable advice, encouragement and assistance through the course of his study and in preparation of his thesis. Special thanks for prof. Keith Crews for providing the opportunity for collaboration of industrial partner (Energy Australia) for this study. I wish to sincerely thank Dr. Fook Choon Choi for his encouragement, help and support and especially for his exceptional personality and source of inspiration as a role model for my professional and personal development. The author also gratefully acknowledges the financial assistance provided by the University of Technology Sydney and Centre for Built Infrastructure Research (CBIR) of the Faculty of Engineering, UTS and Energy Australia an industrial partner.

Furthermore, the author would like to thank the Structures Laboratory staff for their help in the experimental and field work. Special thanks must also go to Rami Haddad, David Hooper, David Dicker, Peter Brown and Richard Turnell. I wish to sincerely thank for the members of timber pole project; Dr. Ulrike Dackermann, Saad, Bahram and Ning. To friends and/or colleagues at UTS, the author wishes to express his gratitude. Special thanks must go to Behnam, Ali, Nassim, Mohsen, Muhammad and others who shared their time and friendship with the author

The administrative and the support staff at UTS Faculty of Engineering and IT, Phyllis Agius, Craig Shuard, Van Lee and the IT support team for performing an excellent job in keeping the show running.

To My **Mum** and **Dad**

List of Publications Based on This Research

Journal Articles

1. Zad, A., Li, J., Samali, B. (2013), 'Limitation of surface non-destructive tests on timber utility poles', *In preparation*
2. Zad, A., Li, J., Samali, B. (2013), 'Comprehensive investigation of dispersive wave on timber utility poles', *In preparation*

Book Chapters

3. Zad, A., Li, J., Samali, B., & Crews, K. (2011), 'Finite element evaluation of non-destructive testing methods for embedded timber poles in service', *Incorporating Sustainable Practice in Mechanics and Structures of Materials*, Taylor & Francis Group, London, pages 909-914

Conference Papers

4. Zad, A., Li, J., Crews, K. & Samali, B. (2010), 'Determination of embedment depth of timber poles and piles using non-destructive evaluation (NDE) techniques', *Proceedings of the 13th International Conference on Structural Faults + Repair*, 15-17 June 2010, Edinburgh, Scotland, Paper #TB CREWS - SFR, (published on CD).

Guidelines

5. Li, J., Dackermann, U., Zad, A., Subhani, S., Samali, B. & Crews, K., 'Recommended Best Practice for Assessment of Timber Structures using Surface Stress Wave Methods', RILEM, *In press*.

Table of Contents

CHAPTER 1	1
1 INTRODUCTION.....	1
1.1 Background	1
1.2 Research Scope.....	3
1.3 Research Objectives	4
1.4 Summary of Contributions	5
1.5 Outline of the Thesis	7
CHAPTER 2	9
2 LITERATURE REVIEW.....	9
2.1 Timber poles in Australia	9
2.2 Types of degradation and location of timber piles and utility poles	11
2.3 Conventional methods for assessment of timber structures	12
2.3.1 Visual inspection method.....	12
2.3.2 Probing.....	13
2.3.3 Sounding	13
2.3.4 Drilling and coring	13
2.4 Non-destructive evaluation methods for timber structures	14
2.5 Stress wave testing based methods.....	15
2.5.1 Sonic Echo (SE) method.....	15
2.5.2 Impulse response method.....	16
2.5.3 Bending Wave method.....	19
2.5.4 Ultraseismic Method	22
2.5.5 Parallel Seismic (PS).....	23
2.5.6 Borehole radar	26
2.6 Review of stress wave propagation in solids.....	28
2.6.1 Wave propagation in an elastic half-space.....	28

2.6.2	Longitudinal wave propagation in thin rods	31
2.6.3	Flexural wave propagation in thin rod	33
2.7	Signal processing for stress wave methods	36
2.7.1	The discrete and fast Fourier transform	36
2.7.2	Short-Kernel Method (SKM).....	39
2.7.3	Wavelet transform.....	41
2.8	Finite element modelling of wave propagation in cylindrical pile/pole.....	42
2.9	Research gaps identified.....	44
2.10	Summary.....	46
CHAPTER 3		49
3 FINITE ELEMENT MODELLING OF TIMBER POLE WITH/WITHOUT SOIL EMBEDMENT		49
3.1	Introduction	49
3.2	Numerical Modelling of timber poles	50
3.2.1	Finite Element Modelling of Intact Beam.....	50
3.2.2	Consideration of various boundary conditions	51
3.2.3	Material properties and geometry	53
3.3	Simulation of wave propagation in timber pole	54
3.3.1	Simulation of impact loading.....	56
3.3.2	Stress wave propagation through the pole under impact load	56
3.4	Behaviour of wave propagation in timber pole	57
3.5	Effect of types of impact and their location on timber pole	59
3.6	Application of Sonic Echo/Impulse Response test on timber pole	62
3.6.1	Velocity calculation	62
3.6.2	Embedded length determination	65
3.7	Application of Bending Wave Test on timber pole.....	66
3.7.1	Velocity calculation	67

3.7.2	Embedded length determination	72
3.7.3	Velocity calculation and length determination of filtered results	74
3.8	Application of Ultraseismic test impact on timber pole.....	77
3.8.1	Velocity calculation	77
3.8.2	Embedded length determination	81
3.8.3	Alternative impact location.....	82
3.9	Preliminary damage identification of timber pole.....	84
3.10	Summary.....	87
CHAPTER 4	90
4	EXPERIMENTAL INVESTIGATION OF TIMBER UTILITY POLES.....	90
4.1	Introduction	90
4.2	Test Equipment.....	90
4.2.1	Impact hammer.....	90
4.2.2	Accelerometers.....	91
4.2.3	Signal Conditioning and computer	93
4.2.4	Laboratory testing frame	94
4.3	Testing Scenarios	96
4.3.1	Testing Procedure.....	96
4.3.2	Test specimens	99
4.3.3	Different types of testing.....	100
4.3.4	Damage scenario induced for timber pole	103
4.4	Test set-up for Sonic Echo and Impulse Response Method.....	104
4.5	Test Set-Up for Bending Wave Method.....	108
4.6	Controlled field tests	110
4.7	Field tests on decommissioned utility poles.....	112
4.8	Classification of damage of field utility timber poles	113
4.9	Soil samples.....	116

4.10	Summary.....	119
CHAPTER 5		121
5	ANALYSIS AND DISCUSSION OF LABORATORY AND FIELD TESTS	121
5.1	Introduction	121
5.2	Sonic Echo (SE) test.....	122
5.2.1	Velocity calculation	123
5.2.2	Length Estimation	134
5.3	Impulse Response (IR) test.....	144
5.3.1	Impact at the top.....	144
5.3.2	Impact at the middle.....	148
5.4	Bending Wave test.....	149
5.4.1	Application of SKM for the calculation of phase velocity	151
5.4.2	Velocity calculation	153
5.4.3	Length Estimation	157
5.5	Ultraseismic test	159
5.5.1	Velocity calculation	159
5.5.2	Length Estimation	172
5.6	Effects of damage scenarios on timber pole in laboratory	173
5.7	Controlled field tests	175
5.7.1	Sonic Echo/Impulse response test at the middle	175
5.7.2	Ultraseismic test at the middle	180
5.8	Field tests of decommissioned utility poles.....	184
5.8.1	Sonic Echo (SE) test impact at top.....	185
5.8.2	Sonic Echo/Impulse Response test impact at the middle.....	187
5.8.3	Ultraseismic test impact at the top	189
5.9	Summary	192

CHAPTER 6	196
6 CONCLUSION AND RECOMMENDATION	196
6.1 Summary	196
6.2 Concluding remarks	198
6.3 Recommendation of future study	202
REFERENCES.....	203
APPENDIX A.....	208
TIMBER POLE AUTOPSY	208
APPENDIX B	225
RESULTS OF IMPULSE RESPONSE (IR) TEST	225
APPENDIX C	228
RESULTS OF ULTRASEISMIC TEST.....	228

List of Figures

Figure 2.1 Three principal axes of wood with respect to grain direction and growth rings (Green, Winandy et al. 1999).....	11
Figure 2.2 Possible decay patterns in underground sections of utility poles (Nguyen, Foliente et al. 2004).....	12
Figure 2.3 Schematic principle of SE testing.....	16
Figure 2.4 Schematic principle of IR testing	17
Figure 2.5 Schematic principle of Bending Wave testing	21
Figure 2.6 Schematic principle of Ultraseismic testing	23
Figure 2.7 Parallel Seismic setup	24
Figure 2.8 Parallel Seismic data and velocity lines.....	25
Figure 2.9 Borehole Radar system.....	27
Figure 2.10 A thin prismatic rod with coordinate x and displacement u of a section.....	31
Figure 2.11 A differential element of a thin rod undergoing transverse motion due to a vertical impact.....	33
Figure 2.12 Dispersion relation for different theories (After Graff, 1975).....	36
Figure 2.13 Kernel shifted along signal	40
Figure 2.14 Consideration of damage identification for determination of the underground length of a timber pole.....	45
Figure 3.1 The geometric properties of SOLID45 (ANSYS Inc 2011).....	50
Figure 3.2 A typical set-up for the embedded timber pole	52
Figure 3.3 A typical FE model of embedded timber pole.....	52
Figure 3.4 The geometric properties of CONTACT178 (ANSYS Inc 2011).....	53
Figure 3.5 An example of applied impact loading in the transient dynamic analysis. ...	56
Figure 3.6 Geometry of the model and location of the sensors placed on the timber pole	57
Figure 3.7 Velocity results in y direction under free-free condition.....	58
Figure 3.8 Velocity results in y direction for patch sensors with same distance under free-free condition.....	58
Figure 3.9 Cross section of FE modelling.....	59
Figure 3.10 Stress wave velocity in y direction under free end condition and different loading condition by impact from the top for accelerometer 1	60

Figure 3.11 Stress wave velocity in y direction under free end condition and different loading condition by impact from the top for accelerometer 4	60
Figure 3.12 Stress wave velocity in x direction under free end condition by impact from the top for accelerometers 4 and 13	61
Figure 3.13 Stress wave velocity in x direction under free end condition by impact at the edge for accelerometers 4 and 13	61
Figure 3.14 Geometry of the model and location of the sensors placed on the timber pole for Sonic Echo test	63
Figure 3.15 Acceleration results in y direction for patch sensors under 5 pull out conditions	63
Figure 3.16 Effect of different embedded lengths for velocity calculation	64
Figure 3.17 Acceleration result of 5m timber pole under 2 nd pull out condition.....	64
Figure 3.18 Acceleration result of 5m timber pole under 4 th pull out condition.....	65
Figure 3.19 Geometry of the model and location of the sensors placed on the timber pole for Bending Wave test.....	67
Figure 3.20 SKM coefficient plot at 410 Hz of 5 m timber pole under 1 st pull out for sensors 1,2 and 4.	69
Figure 3.21 SKM coefficient plot at 410 Hz of 5 m timber pole under 1 st pull out for sensor 1	69
Figure 3.22 Bending wave velocity for different kernel frequencies of 5 m timber pole under different boundary conditions using sensor 1 as a reference a) using sensor 2 as a reflection wave, b) using sensor 3 as a reflection wave, c) using sensor 4 as a reflection wave	71
Figure 3.23 Bending wave velocity for different kernel frequencies of 5 m timber pole under different boundary conditions using sensor 2 as a reference, a) using sensor 3 as a reflection wave, b) using sensor 4 as a reflection wave.....	72
Figure 3.24 Embedded length determination for different kernel frequencies of 5 m timber pole under 1 st pull out condition.	73
Figure 3.25 Embedded length determination for different kernel frequencies of 5 m timber pole under 3 rd pull out condition.....	73
Figure 3.26 Embedded length determination for different kernel frequencies of 5 m timber pole under 8 layer soil condition.	74

Figure 3.27 Bending wave velocity for different kernel frequencies of 5 m timber pole under 3 rd pull out condition using filtered results.	75
Figure 3.28 Embedded length determination for different kernel frequencies of 5 m timber pole under 3 rd pull out condition after filtering.	75
Figure 3.29 Embedded length error for different frequencies of 12 m timber pole under 2m embedment condition using continues wave length and short kernel method.....	76
Figure 3.30 Acceleration results for selected sensors in y direction under 3 rd pull out condition using Ultraseismic method a) arrival wave and b) reflection wave	79
Figure 3.31 Acceleration results in y direction under 3 rd pull out condition using Ultraseismic method.....	79
Figure 3.32 Acceleration results for selected sensors in y direction under 5 th pull out condition using ultraseismic method a) arrival wave and b) reflection wave.....	80
Figure 3.33 Acceleration results in y direction under 5 th pull out condition using Ultraseismic method.....	81
Figure 3.34 Effect of soil in stress wave velocity	81
Figure 3.35 Length estimation using sensors above the soil level under 3 rd pull out condition using Ultraseismic method in 2D graph.....	82
Figure 3.36 Geometry of the model and location of the sensors placed on the timber pole for Ultraseismic test impacted at the middle	83
Figure 3.37 Acceleration-time history of 12m timber pole under 5 th pull out condition using Ultraseismic method with impact at the middle	83
Figure 3.38 Velocity results for decay type 1 under three different damage scenarios; 1S, 1M and 1L.....	84
Figure 3.39 Comparison of the free end test with external decay and without decay	85
Figure 3.40 Side view of a damage inflicted in timber pole identical to the laboratory case.....	86
Figure 3.41 Acceleration-time history results for intact and damaged pole of 5m timber pole for sensor 3	86
Figure 3.42 Acceleration-time history results for intact and damaged pole of 5m timber pole for sensor 6	87
Figure 4.1 Impact hammer (a) stiff tip, (b) soft tip	91
Figure 4.2 Typical response of hammer impact.....	91

Figure 4.3 Testing accelerometers (a) piezoelectric accelerometer - model PCB 356A08, (b) piezoelectric accelerometer - model PCB 337A26, (c) piezoresistive accelerometer chip ADXL320 (d) piezoresistive accelerometer with housing.....	92
Figure 4.4 Accelerometers mounted to the a) steel beam and b) timber beam by screws	92
Figure 4.5 Calibration of the accelerometers using a shake table.....	93
Figure 4.6 (a) Multi-channel signal conditioner - model PCB 483B03 and (b) DC power supply.....	93
Figure 4.7 A personal computer for laboratory and field testing.....	94
Figure 4.8 Steel frame used as a container.....	95
Figure 4.9 Using scaffold and scissor lift to build and access the top of the frame.....	95
Figure 4.10 Testing procedure: (a) setting up of equipment, (b) mounting of accelerometer, (c) attached bracket and accelerometers and (d) execution of the test. ..	98
Figure 4.11 Laboratory free-free test for steel beam.....	99
Figure 4.12 Laboratory free-free test set-up for timber beam.....	100
Figure 4.13 a) location of the accelerometers on steel and timber beam specimens, b) laboratory set-up for NDT method under free-end conditions with timber pole specimen	100
Figure 4.14 Test set-up of bedrock condition in laboratory.....	101
Figure 4.15 Filing the sand a) into the buckets and b) into the frame.....	102
Figure 4.16 The laboratory set-up for NDT methods under embedded conditions for a timber pole	103
Figure 4.17 The laboratory set-up for NDT methods using pull out to simulate various embedment depths (timber beam specimen).....	103
Figure 4.18 Side view of a typical damage inflicted in timber pole in laboratory.....	104
Figure 4.19 Impact bracket mounted to the side of a timber pole to provide a surface for longitudinal impact excitation.....	104
Figure 4.20 Impact bracket mounted to the side of a a) steel pole b) timber beam to provide a surface for longitudinal impact excitation.....	105
Figure 4.21 Test set-up for free-free testing of (a) and (b) laboratory testing and (c) to (e) field testing.	106
Figure 4.22 Test set-up of embedded testing in laboratory.....	107
Figure 4.23 Test set-up of embedded testing in the field.....	107

Figure 4.24 Schematic test set-up of free-free BW tests for (a) laboratory testing and (b) field testing.....	108
Figure 4.25 Schematic and photo of test set-up of embedded BW tests for laboratory testing.....	109
Figure 4.26 Schematic and photo of test set-up of embedded BW tests for field testing.....	109
Figure 4.27 Location of the Mason Park (courtesy of Google Maps)	110
Figure 4.28 Location of timber poles at Mason Park.....	110
Figure 4.29 Timber poles before installation in Mason Park.....	111
Figure 4.30 Timber poles after installation in Mason Park.....	111
Figure 4.31 Cross sections of pole No 288	115
Figure 4.32 Compaction test equipment	117
Figure 4.33 Filling the compaction mould with soil	118
Figure 4.34 Compacted soil with mould after compaction completed	118
Figure 4.35 Compaction curve of soil sample in Mason Park	118
Figure 5.1 Test set-up for (a) free-free testing and (b) embedded testing condition	123
Figure 5.2 Velocity calculation of steel beam (free end condition).....	124
Figure 5.3 Velocity calculation of timber beam (free end condition).....	124
Figure 5.4 Velocity calculation of timber pole (free end condition).....	125
Figure 5.5 Minimum, maximum and average longitudinal wave velocity for steel beam	125
Figure 5.6 Minimum, maximum and average longitudinal wave velocity for timber beam	126
Figure 5.7 Minimum, maximum and average longitudinal wave velocity for timber pole	126
Figure 5.8 The three principal axes of wood with respect to grain direction and growth rings (Kretschmann 2010).....	133
Figure 5.9 Direction of load in relation to direction of annual growth rings: 90° or perpendicular (R), 45°, 0° or parallel (T) (Kretschmann 2010).....	133
Figure 5.10 Relationship of fibre orientation (O–O) to different axes, as shown by the schematic of wood specimens containing straight grain and cross grain. Specimens A through D have radial and tangential surfaces; E through H do not. Specimens A and E	

contain no cross grain; B, D, F, and H have spiral grain; C, D, G, and H have diagonal grain (Kretschmann 2010).....	133
Figure 5.11 Schematic of fibre orientation and sensor location	134
Figure 5.12 Stress wave velocity in timber material with knots	134
Figure 5.13 Acceleration-time history result of a 5m timber pole under free-free and bedrock conditions	136
Figure 5.14 Percentage errors for different tests estimating the length of the steel beam for sensor 1 (located on top of the specimen).....	137
Figure 5.15 Percentage errors for different tests estimating the length of the steel beam for sensor 2 (located 1m below the top of the specimen)	138
Figure 5.16 Percentage errors for different tests estimating the length of the steel beam for sensor 3 (located 1.5m below the top of the specimen)	138
Figure 5.17 Percentage errors for different tests estimating the length of the steel beam for sensor 4 (located 2m below the top of the specimen)	139
Figure 5.18 Percentage errors for different tests on the timber beam for Sensor 1(located on top of the specimen)	140
Figure 5.19 Percentage errors for different tests on the timber beam for Sensor 2 (located 1m below the top of the specimen)	140
Figure 5.20 Percentage errors for different tests on the timber beam for Sensor 3(located 1.5m below the top of the specimen)	141
Figure 5.21 Percentage errors for different tests on the timber beam for Sensor 4(located 2m below the top of the specimen)	141
Figure 5.22 Percentage errors for different tests on the timber pole for Sensor 1(located on top of the specimen)	142
Figure 5.23 Percentage errors for different tests on the timber pole for Sensor 2 (located 1m below the top of the specimen)	143
Figure 5.24 Percentage errors for different tests on the timber pole for Sensor 3(located 1.5m below the top of the specimen)	143
Figure 5.25 Percentage errors for different tests on the timber pole for Sensor 4(located 2m below the top of the specimen)	144
Figure 5.26 FRFs of different sensors of (a) free-free and (b) embedded laboratory IE testing of a timber beam with impact from top.	146
Figure 5.27 Percentage errors for different tests on steel beam.....	147

Figure 5.28 Percentage errors for different tests on timber beam.....	147
Figure 5.29 Percentage errors for different tests on timber pole.....	148
Figure 5.30 FRFs of different sensors of (a) free-free and (b) embedded laboratory IR testing of a timber beam with impact from the side.....	150
Figure 5.31 Schematic test set-up of embedded BW tests for laboratory testing.....	151
Figure 5.32 An example of Raw signals vs SKM plots at specific kernel frequency...	152
Figure 5.33 An example of the frequency response function (FRF) from a timber pole under 3rd pull out condition.....	152
Figure 5.34 SKM plot at frequency of 725 Hz: Timber pole under 3rd pull out condition.....	153
Figure 5.35 Bending wave velocity for different kernel frequencies of 5 m timber pole under different boundary conditions using sensors 1 and 2.....	155
Figure 5.36 Bending wave velocity for different kernel frequencies of 5 m timber pole under different boundary conditions using sensors 1 and 3.....	155
Figure 5.37 Bending wave velocity for different kernel frequencies of 5 m timber pole under different boundary conditions using sensors 1 and 4.....	156
Figure 5.38 Bending wave velocity for different kernel frequencies of 5 m timber pole under different boundary conditions using sensors 2 and 3.....	156
Figure 5.39 Bending wave velocity for different kernel frequencies of 5 m timber pole under different boundary conditions using sensors 2 and 4.....	157
Figure 5.40 Percentage errors for different kernel frequencies of the timber pole under 1 st pull out condition.....	158
Figure 5.41 Percentage errors for different kernel frequencies of the timber pole under 3 rd pull out condition.....	158
Figure 5.42 Percentage errors for different kernel frequencies of the timber pole under 5 th pull out condition.....	159
Figure 5.43 Acceleration results for all sensors in y direction under free-free condition for 5 m steel beam using Ultraseismic method.....	161
Figure 5.44 Acceleration results for selected sensors in y direction under free-free condition for 5 m steel beam using Ultraseismic method (sensors at 0, 1, 1.5 and 2 m from the top).....	162
Figure 5.45 Acceleration results for all sensors in y direction under free-free condition for the 5 m timber beam using Ultraseismic method.....	163

Figure 5.46 Acceleration results for selected sensors in y direction under free-free condition for the 5 m timber beam using Ultraseismic method (sensors at 0, 1, 1.5 and 2 m from the top).....	164
Figure 5.47 Acceleration results for all sensors in y direction under free-free condition for the 5 m timber pole using Ultraseismic method.....	166
Figure 5.48 Acceleration results for selected sensors in y direction under free-free condition for the 5 m timber pole using Ultraseismic method (sensors at 0, 1.5 , 3 and 5m from the top).....	167
Figure 5.49 Acceleration results for selected sensors in y direction under 1 layer soil condition for the 5 m timber pole using Ultraseismic method (sensors at 0, 1, 1.5 , 2 and 3m from the top).....	170
Figure 5.50 Acceleration results for selected sensors in y direction under 1 layer soil condition for the 5 m timber pole using Ultraseismic method (sensors at 0, 1.5 and 3m from the top).....	171
Figure 5.51 Embedded length determination of 5 m timber pole under different boundary conditions using sensors at 0, 1.5 and 3m from the top.....	173
Figure 5.52 Schematic set up for intact and damaged timber pole in laboratory.....	174
Figure 5.53 Acceleration results for an intact and damaged timber pole under free-free condition for sensor 3.....	174
Figure 5.54 Acceleration results for an intact and damaged timber pole under free-free condition for sensor 4.....	174
Figure 5.55 Test set-up of embedded testing in Mason Park.....	175
Figure 5.56 FFT result of the timber pole with 1 m of embedment.....	176
Figure 5.57 FFT result of the timber pole with 1.5 m of embedment.....	176
Figure 5.58 FFT result of the timber pole with 2 m of embedment.....	176
Figure 5.59 Velocity calculation of pole 8 with 1m embedded length	177
Figure 5.60 Velocity calculation of pole 14 with 1.5m embedded length	177
Figure 5.61 Velocity calculation of pole 1 with 2m embedded length	177
Figure 5.62 Percentage errors for different sensors estimating the length of the timber pole for 1 m embedment	178
Figure 5.63 Percentage errors for different sensors estimating the length of the timber pole for 1.5 m embedment	179

Figure 5.64 Percentage errors for different sensors estimating the length of the timber pole for 2 m embedment	179
Figure 5.65 Acceleration results for all sensors in y direction under 1m embedment using Ultraseismic method (Pole 8)	181
Figure 5.66 Acceleration results for all sensors in y direction under1.5m embedment using Ultraseismic method (Pole 14)	182
Figure 5.67 Acceleration results for all sensors in y direction under1.5m embedment using Ultraseismic method (Pole 1)	183
Figure 5.68 Percentage errors for different embedment conditions estimating the length of the timber pole using ultraseismic method	184
Figure 5.69 Test set-up for free-free testing of field testing in Horsham, (a) impact from the side, (b) impact from the end	185
Figure 5.70 FFT result of the timber pole under free-free condition (Pole3-293) impact from location 3(at the top).....	186
Figure 5.71 FFT result of the damaged timber poles under free-free condition (Pole288, 183 and 299) impact from location 3(at the top).....	187
Figure 5.72 Acceleration-time history of timber pole under free-free condition (Pole293) impact from location 3(at the top).....	188
Figure 5.73 Acceleration-time history of timber pole under free-free condition (Pole288) impact from location 3(at the top).....	188
Figure 5.74 FFT result of the timber pole under free-free condition (Pole293) impact from location 1(at the middle).....	189
Figure 5.75 FFT result of the timber pole under free-free condition (Pole 288) impact from location 1(at the middle).....	189
Figure 5.76 Results of the timber pole under free-free condition using Ultraseismic method (Pole293) impact from location 3(at the top).....	190
Figure 5.77 Results of the timber pole under free-free condition using Ultraseismic method (Pole288) impact from location 3(at the top).....	191
Figure 6.1 a) Stable case b) Unstable case.....	198

List of Tables

Table 2.1 Estimated quantities of poles in-service throughout Australia in 2004 (Kent 2006)	9
Table 3.1 Material properties used in the FE model	54
Table 3.2 Length determination of a 5m timber pole under different embedded lengths using the first four sensors with Sonic Echo test	66
Table 3.3 Material properties used in guided wave solution (Subhani 2013).....	78
Table 3.4 Decay pattern type 1 modelling using FE	84
Table 4.1 Field test details	112
Table 4.2 Timber pole classifications based on the existing defects at Horsham.....	113
Table 4.3 Defect description of different timber poles at Horsham.....	116
Table 5.1 The coefficient of variation of the velocity calculation for repeated tests of steel beam under different conditions.	130
Table 5.2 The coefficient of variation of the velocity calculation for repeated tests of timber beam under different conditions.	131
Table 5.3 The coefficient of variation of the velocity calculation for repeated tests of timber pole under different conditions.....	132
Table 5.4 Calculation of Characteristic Impedance for different materials	135
Table 5.5 Timber pole classifications based on the existing defects at Horsham.....	186