

ENGINEERING PROPERTIES OF POLYSTYRENE AGGREGATE CONCRETE

BENJAMIN ASHONG SABAA

B. Sc. (Hons), UST, Kumasi, M. Eng. Sc. UNSW, Sydney

**Submitted for the Degree of
Doctor of Philosophy**

**Faculty of Engineering
University of Technology, Sydney**

1998

CERTIFICATE

I certify that this thesis has not already been submitted for any degree and is not being submitted as part of candidature for any other degree.

I also certify that the thesis has been written by me and that any help that I have received in preparing this thesis, and all sources used, have been acknowledged in this thesis.

Signature of Candidate

Production Note:
Signature removed prior to publication.

.....

ABSTRACT

The project reported in this thesis was concerned with the utilization of re-cycled polystyrene granulates as lightweight aggregate for use in concrete. A manufacturing process for the conversion of polystyrene waste from the packaging industry into chemically coated expanded polystyrene aggregate was developed by Building Systems Technology (BST) Pty. Ltd. When the treated polystyrene aggregates are incorporated into fresh mortar or concrete they are uniformly and evenly distributed in the cement paste or the mortar matrix.

The polystyrene aggregate produced by BST was used to establish the workability, strength, deformation, bond strength, and the functional properties of the concrete. The properties of the concretes made with the polystyrene aggregate were compared with concretes made with normal weight aggregates of equivalent mix proportions using General Purpose Portland (Type GP) cement.

It was found that it is generally feasible to manufacture structural grade lightweight concrete from treated re-cycled polystyrene aggregate. No reduction was observed in the compressive and tensile strengths, and the modulus of elasticity of concretes made with the polystyrene aggregate, and cured in water over a period of about one year. The maximum cylinder compressive strength of concrete made with the treated polystyrene aggregate satisfied the strength requirement of medium strength structural reinforced concrete.

This investigation has shown that structural grade polystyrene aggregate concrete having saturated surface-dry density of 1800 kg/m³ to 2400 kg/m³ can be produced with cylinder compressive strength up to 32 MPa. The test results have shown that, for a stress/strength ratio of 30% of the 28-day cylinder compressive strength, the creep strain of polystyrene aggregate concrete compares well with concrete made with normal weight aggregates. The functional properties such as impact resistance and freezing and thawing durability of concrete is improved when polystyrene aggregate is incorporated.

From the conclusions derived, design recommendations are suggested. Limitations of the investigation and suggestions for future work are presented.

ACKNOWLEDGMENTS

The author wishes to express his thanks and gratitude to Dr. R. Sri Ravindrarajah for his valuable advice, continuous guidance, encouragement and supervision of this work. The author acknowledges gratefully the Faculty of Engineering, University of Technology, Sydney for making facilities available to me to undertake this research. The author also thanks the Academic Staff of the School of Civil Engineering who offered many helpful suggestions, comments, and criticisms during seminar presentations of the finding of the research.

The author also acknowledges the assistance of Mr I Hutchings and the Technical Staff of the Civil Engineering Laboratories, in particular Mr W. Howse. The author also extends his thanks to Mr J. Grove and the Staff of the Civil and Mechanical Engineering Workshop for the valuable assistance in the fabrication of some of the testing equipments.

A deep sense of gratitude is due to the Australian Research Council for the award of an Australian Postgraduate Research Award (Industry) made possible through the sponsorship and support of the project by Building Systems Technology (BST) Pty. Ltd. The author is also grateful to BST (Australia) Pty. Ltd. for providing the polystyrene aggregate used in the research project.

Finally to family and friends the author acknowledges their patience and encouragement, in particular, Esther Dedei Armah who assisted with some preparation and testing, and typing of the manuscript.

DEDICATION

*This work is dedicated to the memory
of my father SAMUEL DOKU SABAA.*

TABLE OF CONTENTS

ABSTRACT	i
ACKNOWLEDGMENT	ii
DEDICATION	iii
TABLE OF CONTENTS	iv
LIST OF FIGURES	xiv
LIST OF TABLES	xxvii
LIST OF PLATES	xxxii
NOTATIONS	xxxiii
ABBREVIATIONS	xxxiv
CHAPTER 1 INTRODUCTION	1
1.1 General introduction	1
1.2 Purpose and scope	2
CHAPTER 2 REVIEW OF PREVIOUS INVESTIGATION INTO POLYSTYRENE AGGREGATE CONCRETE	5
2.1 Introduction	5
2.2 Expanded polystyrene in concrete	6
2.2.1 Coated polystyrene beads in concrete	6
2.2.2 Waste polystyrene chips in concrete	8
2.2.3 Uncoated polystyrene beads in concrete	8
2.3 Applications of polystyrene concrete	9
2.3.1 Concrete block making	9
2.3.2 Structural applications	9
2.3.3 Other applications	10
CHAPTER 3 EXPERIMENTAL PROGRAMME	11
3.1 Scope of experimental investigation	11
3.2 Details of concrete materials	11
3.2.1 Portland cement	11

3.2.2	Aggregates	11
3.3	Details of Concrete Mixes	13
3.4	Preparation of test specimens for hardened concrete properties study	15
3.4.1	Mixing procedure	15
3.4.2	Concrete specimen for strength and creep studies	21
3.4.3	Concrete prisms for shrinkage and durability studies	22
3.4.4	Concrete specimens for bond strength studies	23
3.4.5	Impact resistance test specimens	23
3.5	Apparatus	23
3.5.1	Compression creep testing rigs	23
3.5.2	Impact strength testing equipment	27
3.5.3	Pulse velocity measurement apparatus	28
3.5.4	Resonant frequency apparatus	28
3.5.5	Ancillary equipment	30
3.6	Experimental investigation	32
3.6.1	Determination of the strength properties of polystyrene aggregate concrete	32
3.6.2	Deformations of polystyrene aggregate concrete	33
3.6.3	Creep of polystyrene aggregate concrete in compression	33
3.6.4	Drying shrinkage of polystyrene aggregate concrete	35
3.6.5	Bond development with reinforcing steel studies	37
3.6.6	Impact resistance of polystyrene aggregate concrete	39
3.6.7	Freeze-thaw resistance of polystyrene aggregate concrete	40
3.7	General comments	41
CHAPTER 4	PROPERTIES OF FRESH POLYSTYRENE AGGREGATE CONCRETE	42
4.1	Workability of concrete	42
4.1.1	Introduction	42

4.1.2	Factors influencing concrete workability	42
4.1.2.1	Influence of type and property of aggregate on workability	43
4.1.2.2	Influence of mix proportions on workability	43
4.1.2.3	Influence of time and temperature on workability	44
4.1.3.	Workability of lightweight aggregate concrete	45
4.1.3.1	Measurement of lightweight aggregate concrete workability	46
4.2	Workability of polystyrene aggregate concrete	47
4.2.1.	Evaluation of the DIN Compaction Test	47
4.2.1.1	Effect of size of compaction test container	47
4.2.1.2	Effect of method of compaction on compaction index	51
4.2.1.3	Optimising compaction by ramming	54
4.2.2	Effect of mix composition on PAC workability	58
4.2.2.1	Effect of polystyrene aggregate content on workability	58
4.2.2.2	Influence of unit weight on workability	60
4.2.2.3	Effect of fine aggregate content on workability	63
4.2.2.4	Effect of aggregate grading on workability	65
4.2.3	Sensitivity of the compaction test to aggregate grading	68
4.2.4	Sensitivity of the compaction test to binder type	68
4.2.5	Classification of workability of PAC	69
4.2.5.1	Visual observation and classification of workability	71
4.2.5.2	Relating air bubble escape time and visual observation	73
4.2.6	Comparison of workability tests	75
4.2.6.1	Slump of PAC compared to that of reference concrete	79
4.2.6.2	Relation between compaction test and slump test	79
4.2.7	Variability and repeatability of DIN compaction test	81
4.3	Unit weight of fresh PAC	85
4.3.1	Relation between PA replacement levels and unit weight	85
4.3.2	Relation between PA addition levels and unit weight	87
4.3.3	Statistical significance of factors influencing unit weight	91
4.3.3.1	The significance of the influence of cement content on unit weight	92
4.3.3.2	The significance of the influence of water/cement ratio on unit weight	97
4.3.3.3	The significance of the influence of method of PA incorporation on unit weight	100
4.3.3.4	The significance of the influence of type of reference mix on unit weight	100

4.3.4	Regression model for predicting unit weight of PAC with normal weight coarse aggregate	105
4.3.5	Regression model for predicting unit weight of PAC without normal weight coarse aggregate	108
4.3.6	Prediction intervals	111
4.3.7	Simplified regression models	114
4.4	Concluding remarks	116
4.4.1	General	116
4.4.2	Conclusions	116
 CHAPTER 5 STRENGTH AND ELASTIC DEFORMATION OF POLYSTYRENE AGGREGATE CONCRETE		 118
5.1	Introduction	118
5.2	Theoretical principles	118
5.2.1	Concrete as a composite material	118
5.2.2	Mathematical models for modulus of concrete	119
5.2.3	The influence of aggregate type on the strength of concrete	125
5.3	Strength of polystyrene aggregate concrete	130
5.3.1	Compressive strength	130
5.3.1.1	Influence of concrete density on compressive strength	130
5.3.1.2	Effect of age on compressive strength	148
5.3.2	Prediction of compressive strength	153
5.3.2.1	Limiting compressive strength	155
5.3.3	Effect of specimen shape on compressive strength	159
5.3.4	Tensile strength of concrete	159
5.3.4.1	Influence of concrete density on indirect tensile strength	161
5.3.4.2	Relation between concrete density and indirect tensile strength	166
5.3.4.3	Comparison of relative indirect tensile strength with relative compressive strength	166
5.3.5	Relation between indirect tensile strength and cylinder compressive strength	168
5.3.6	Relation between indirect tensile strength and cube compressive strength	167

5.3.7	Relation between flexural strength and compressive strength	172
5.3.8	Theoretical prediction of compressive strength	174
5.4	Elastic deformation in compression	179
5.4.1	Stress-Strain relation of PAC	179
5.4.1.1	Strain at peak stress	183
5.4.1.2	Equation for stress-strain relation for PAC	186
5.4.2	Modulus of elasticity	189
5.4.2.1	Expressions relating modulus of elasticity and compressive strength	193
5.4.2.2	Relation between the moduli for PAC and reference concrete	199
5.4.2.3	Expressions relating modulus of elasticity to strength and density	199
5.4.2.4	Relation between static and dynamic modulus of elasticity	205
5.4.3	Poisson's ratio of PAC	207
5.4.4	Theoretical estimation of the modulus of elasticity	213
5.5	Conclusions	218
CHAPTER 6	TIME-DEPENDENT DEFORMATIONS OF PAC	220
6.1	Introduction	220
6.2	Theoretical principle	220
6.2.1	The influence of aggregate on the shrinkage and creep of concrete	221
6.3	Shrinkage of polystyrene aggregate concrete	223
6.3.1	Influence of polystyrene aggregate content on drying shrinkage	223
6.3.2	Relation between aggregate content and drying shrinkage	227
6.3.3	Influence of water/cement ratio and aggregate/cement ratio	230
6.3.4	Influence of initial curing duration on shrinkage of PAC	234
6.3.5	Effect of drying condition on shrinkage	239
6.3.6	Ultimate shrinkage of PAC	241
6.3.6.1	Shrinkage-time relation	241
6.3.6.2	Effect of density on ultimate shrinkage	242
6.3.7	Swelling of preshrunk concrete: reversible shrinkage	247
6.3.8	Shrinkage of PAC compared with other lightweight concretes	251

6.4	Creep of polystyrene aggregate concrete	253
6.4.1	Instantaneous strain on loading	254
6.4.2	Creep rate	254
6.4.3	Total creep strain of PAC and lightweight aggregate concrete	256
6.4.3.1	Creep of PAC	256
6.4.3.2	Comparison of creep of PAC with LWAC	259
6.4.4	Influence of cement paste on creep of PAC	260
6.4.5	Influence of strength of concrete on creep of PAC	260
6.4.6	Influence of PA aggregate content on creep of PAC	264
6.4.7	Influence of the age at the time of loading on PAC creep	268
6.4.8	Influence of curing and storage conditions on creep of PAC	270
6.4.9	Prediction of long-term creep	274
6.4.9.1	Prediction of long-term creep from experimental data	276
6.4.9.2	Ultimate creep	280
6.4.9.3	Time to achieve half ultimate creep	280
6.4.10	Creep coefficient	282
6.5	Creep recovery of PAC	285
6.5.1	Components of creep recovery	285
6.5.2	Creep recovery of PAC in relation to density	287
6.5.3	Irrecoverable creep	289
6.5.4	Effect of age of concrete at loading on creep recovery	292
6.6	Prediction of creep and shrinkage from strength, mix composition and environmental conditions	294
6.6.1	Creep and shrinkage prediction by ACI-209R-82	296
6.6.2	Creep and shrinkage prediction by CEB-FIP MC 90	298
6.7	Conclusions	299
CHAPTER 7	BOND PERFORMANCE OF POLYSTYRENE AGGREGATE CONCRETE	302
7.1	Introduction	302
7.1.1	Bond mechanism	302
7.1.2	Types of bond failure	303

7.2	Research into bond strength	304
7.3	Lightweight concrete bond strength research	305
7.4	Bond measurement: requirement in codes and standards	307
7.4.1	BS 4449 method	307
7.4.2	RILEM method	307
7.4.3	ASTM method	308
7.4.4	British Concrete Association method	308
7.5	Bond strength of polystyrene aggregate concrete	309
7.5.1	Failure patterns of pull-out specimens	309
7.5.2	Bond performance in relation to concrete strength	314
7.5.2.1	Relation between bond and tensile strengths	319
7.5.3	Effect of concrete density on bond strength	322
7.5.3.1	Bond performance on the basis of slip	326
7.5.4	Effect of bar direction in relation to casting direction on bond strength	330
7.5.5	Bond performance as a function of concrete cast below bar	331
7.5.6	Development length for bar yield strength	334
7.5.6.1	Development length-compressive strength relation	335
7.5.6.2	Ratio of pull-out test development length to calculated development length	336
7.6	Conclusions	343
CHAPTER 8	FUNCTIONAL PROPERTIES OF POLYSTYRENE AGGREGATE CONCRETE	345
8.1	Introduction	345
8.2	Mechanics of frost action on hardened concrete	345
8.2.1	Coarse aggregate response to frost action	348
8.2.2	Factors controlling frost resistance of concrete	348
8.2.2.1	Air entrainment	349
8.2.2.2	Water/cement ratio	349
8.2.3	Lightweight concrete resistance to freezing and thawing	350

8.2.4	Effect of microspheres on freeze-thaw durability	351
8.2.5	Freeze-thaw test procedures and interpretation	351
8.2.5.1	Merits of standard freezing and thawing tests	352
8.2.5.2	Test method for freeze-thaw investigation	353
8.3	Impact resistance of concrete	353
8.3.1	Requirements of an efficient energy-absorbing material	354
8.3.2	Spring-mass model: hard impact	356
8.3.3	Measurement and characterization of impact resistance	358
8.3.4	Impact testing apparatus	359
8.4	Resistance of PAC to repeated freezing and thawing cycles	361
8.4.1	Properties of fresh concrete	361
8.4.2	Frost response of PAC with normal weight coarse aggregate	361
8.4.2.1	Effect of mix composition on frost resistance	365
8.4.2.2	Effect of freezing and thawing on length and weight changes	367
8.4.2.3	Effect of the density on resistance to freezing and thawing	369
8.4.2.4	Frost response of PAC without normal weight coarse aggregate	373
8.5	Impact resistance of polystyrene aggregate concrete	380
8.5.1	Selection of drop weight	380
8.5.2	Mode of failure under repeated impact	380
8.5.3	Repeatability of test results	383
8.5.4	Relation between impact strength and mix composition	383
8.5.5	Effect of polystyrene aggregate content on impact strength	388
8.5.6	Effect of curing condition on impact strength	390
8.5.7	Effect of curing duration on impact resistance	393
8.5.8	Method of PA inclusion on the impact resistance of concrete	395
8.5.9	Impact resistance of plain PAC compared with fibre reinforced PAC	399
8.6	Conclusions	403
CHAPTER 9	MIX DESIGN OF POLYSTYRENE AGGREGATE CONCRETE	405
9.1	Principle of proposed method	405
9.1.1	Scope of proposed method	405

9.1.2	The mix design process	406
9.2	Relation between the mix constituents and concrete properties	407
9.2.1	Polystyrene aggregate concrete strength and density relation	407
9.2.2	Relation between types of polystyrene aggregate and compressive strength	410
9.2.3	Cement content-strength and water/cement ratio-strength relations	410
9.2.4	Variability of the strength of polystyrene aggregate concrete	412
9.2.5	Water content and workability	413
9.2.6	Estimation of polystyrene aggregate content	413
9.3	Mix design calculations	421
9.3.1	Design of reference concrete	421
9.3.2	PAC mix design based on data obtained in this study	421
9.3.2.1	Alternate method to design reference mix	422
9.4	Trial batch adjustment	423
9.4.1	Adjustment for unit weight	423
9.4.2	Adjustment for strength	427
9.5	Design of polystyrene aggregate concrete without normal weight coarse aggregate	428
9.6	Examples of polystyrene aggregate concrete mix design	430
9.6.1	Specification of required structural polystyrene aggregate concrete	430
9.6.2	Procedure of mix design	430
9.6.2.1	Determination if specified strength and unit weight can be achieved	430
9.6.2.2	Proportioning of reference mix	430
9.6.2.3	Determination of polystyrene aggregate content	432
9.6.3	Adjusting trial mix for density	433
9.6.3.1	Polystyrene aggregate content adjustment	433
9.6.3.2	Normal weight coarse aggregate adjustment	434
9.6.3.3	Adjusting trial mix for strength	434

CHAPTER 10	CONCLUSIONS AND RECOMMENDATIONS	435
10.1	Introduction	435
10.2	Limitation of present work	435
10.3	Conclusions	436
10.3.1	Properties of fresh concrete made with polystyrene aggregate	437
10.3.2	Properties of hardened concrete made with polystyrene aggregate	437
10.4	Design recommendations	440
10.5	Recommendations for further work	441
REFERENCES		442
APPENDICES		
A	Statistical Methods	456
B	Mix proportions of all mixes used for workability and unit weight study	471
C	Regression analysis of unit weight - Tabulated Values	521
D	Workability test results	526

LIST OF FIGURES

Figure	Title	Page
3.1	Samples of expanded polystyrene aggregates	14
3.2	Test specimen for the study of bond strength according to ASTM C234	24
3.3	Test specimen for the bond strength study: modified version of RILEM RC 6.1 pull-out test specimens	24
3.4	Set-up of creep rig used for the creep investigation	26
3.5	Impact strength test equipment	29
3.6	Compressometer arrangement	31
3.7	Profile of reinforcing bar: maximum average spacing 14.0 mm; maximum gap 7.9 mm; minimum average height 1.0 mm; AS 1302 grade 410Y	36
3.8	Set-up of bond strength test according to ASTM C 234	38
3.9	Set-up for bond strength test according to modified RILEM CR 6	38
4.1	Relation between compaction index and water/cement ratio for polystyrene aggregate concrete: unit weight = 1600 kg/m ³	48
4.2	Relation between compaction index and water/cement ratio for polystyrene aggregate concrete: unit weight = 1800 kg/m ³	48
4.3	Effect of diameter of test container on loose unit weight of polystyrene aggregate concrete having different water/cement ratios	50
4.4	Influence of the method of compaction on the unit weight: concrete density = 1650 kg/m ³	52
4.5	Influence of the method of compaction on the unit weight: concrete density = 1800 kg/m ³	52
4.6a	Relation between height change and number of blows of ramming: rammer size = 16 mm diameter; unit weight of concrete = 1650 kg/m ³ .	55
4.6b	Relation between height change and number of blows of ramming: rammer size = 16 mm diameter; unit weight of concrete = 1800 kg/m ³	55
4.6c	Relation between height change and number of blows of ramming: rammer size = 45 mm diameter; unit weight of concrete = 1650 kg/m ³	56

4.6d	Relation between height change and number of blows of ramming: rammer size = 45 mm diameter; unit weight of concrete = 1800 kg/m ³	56
4.6e	Relation between height change and number of blows of ramming: rammer size = 65 mm diameter; unit weight of concrete = 1650 kg/m ³	57
4.6f	Relation between height change and number of blows of ramming: rammer size = 65 mm diameter; unit weight of concrete = 1800 kg/m ³	57
4.7	Relation between compaction index and water/cement ratio (cement content calculated at water/cement ratio of 0.40)	59
4.8	Relation between the slump and water/cement ratio (cement content calculated at water/cement ratio of 0.40)	59
4.9a	Relation between the compaction index and water/cement ratio, for mixes of similar cement paste content	62
4.9b	Relation between the slump of polystyrene aggregate concrete and water/cement ratio, for mixes of similar cement paste content	62
4.10	Combined aggregate grading for polystyrene aggregate concrete having varying fine to coarse aggregate ratio: unit weight 1800 kg/m ³	64
4.11	Effect of fine aggregate content on workability	64
4.12	Combined aggregate grading of polystyrene aggregate concrete having varying percentage fines: nominal unit weight 1800 kg/m ³	66
4.13	Effect of aggregate grading and cement paste content on the workability: unit weight 1800 kg/m ³	66
4.14	Relation between level of fly ash replacement and workability as measured by DIN compaction test and slump test: nominal concrete density = 1800 kg/m ³ ; water/cement ratio of 0.35	70
4.15	Relation between compaction index and compacting effort for concrete with maximum aggregate size of 32 mm.	70
4.16	Relation between workability as determined by visual observation and compaction test	73
4.17	Relation between air bubble escape time, compaction index and visually determined workability	73
4.18	General pattern of relations between the slump, Vebe and DIN compaction tests for polystyrene aggregate concrete mixes of different unit weights and aggregate/cement ratios (by volume)	77

4.19	General pattern of relations between the slump, Vebe and compacting factor tests for polystyrene aggregate concrete mixes of different unit weights and aggregate/cement ratios (by volume).	78
4.20	Ratio of slump of PAC to slump of reference concrete	80
4.21	Relation between compaction index and slump: volume of paste calculated at $w/c = 0.40$	80
4.22	Loss of workability with time (with remixing): unit weight of PAC = 1800 kg/m^3 ; $w/c = 0.40$.	82
4.23	Relation between relative unit weight and percentage level of coarse aggregate replacement with polystyrene aggregate	86
4.24	Regression of relative unit weight on level of coarse aggregate replacement with polystyrene aggregate	86
4.25	Relation between relative unit weight and percentage level of polystyrene aggregate added per unit volume of concrete, with increased water content	88
4.26	Relation between relative unit weight and percentage level of polystyrene aggregate added per unit volume of concrete, with superplasticizer added	88
4.27	Comparison of the method of PA incorporation into reference concrete	90
4.28	Regression of relative unit weight on polystyrene aggregate content	90
4.29	Relation between measured unit weight and calculated unit weight for concretes having different cement contents	94
4.30	Relation between measured unit weight and calculated unit weight for concretes having different water/cement ratio	98
4.31	Influence of the method of polystyrene aggregate incorporation on the unit weight of polystyrene aggregate concrete	102
4.32	Regression of measured unit weight on calculated unit weight for PAC with concrete and mortar reference mixes	103
4.33	Regression of measured unit weight on calculated unit weight for polystyrene aggregate concrete with normal weight coarse aggregate	106
4.34	Diagnostic plot of standardized residuals versus predicted unit weight for PAC with normal weight coarse aggregate	106
4.35	Regression of measured unit weight on calculated unit weight for PAC without normal weight coarse aggregate	109

4.36	Diagnostic plot of standardized residuals versus predicted unit weight for PAC without normal weight coarse aggregate	109
4.37	Estimated line of regression for measured unit weight on calculated unit weight and confidence and prediction limit intervals for PAC with normal weight coarse aggregate: $\alpha = 0.05$.	112
4.38	Estimated line of regression for measured unit weight on calculated unit weight and confidence and prediction limit intervals for PAC without normal weight coarse aggregate: $\alpha = 0.05$	113
5.1	Parallel and series models of concrete	120
5.2	Influence of polystyrene aggregate volume fraction (PAC-Addition) on relative density and relative strength	136
5.3	Influence of polystyrene aggregate volume fraction (PAC-Replacement) on relative density and relative strength	137
5.4	Influence of reference mix composition on 28-day compressive strength (PAC-Addition)	138
5.5	Influence of reference mix composition on 28-day compressive strength (PAC-Replacement)	138
5.6	Relation between void and air content and relative unit weight	140
5.7	Effect of method of polystyrene aggregate inclusion on the relation between relative strength and relative density of 7-day PAC	142
5.8	Effect of method of polystyrene aggregate inclusion on the relation between relative strength and relative density of 28-day PAC	143
5.9	Effect of specimen shape on the relation between relative strength and relative density	144
5.10	Comparison of PAC and incompletely compacted normal weight concrete	145
5.11	Effect of method of PA inclusion on the relative cube strength-density relationship for PAC (mortar reference mix)	146
5.12	Typical relation between compressive strength and age of PAC: Mixes 16CA0, 16CA1, 16CA2, 16CA3, and 16CA4; cement content = 420 kg/m ³ ; w/c = 0.55; aggregate/cement = 4.02	149
5.13	Typical relation between compressive strength and age of PAC without NWCA: Mixes 3MR0 to 3MR70	149

5.14	Relative gain of strength with time in concretes with different densities (mixes 19CA0, 19CA1, 19CA2, 19CA3, and 19CA4)	152
5.15	Influence of concrete density on short-term strength development	152
5.16	Different factors influencing strength of lightweight aggregate concrete	156
5.17	Increase of the 28-day compressive strength as a function of the cement content	158
5.18	Relationship between cylinder and cube compressive strength	160
5.19	Relation between relative indirect tensile strength with concrete density for different mix composition; PAC-Addition	164
5.20	Relation between relative indirect tensile strength with concrete density for different mix composition; PAC-Replacement.	164
5.21	Variation of indirect tensile strength with density for 28-day and 91-day concrete	165
5.22	Relation between 28-day relative indirect tensile strength and relative density	165
5.23	Relation between indirect tensile strength and cylinder compressive strength	170
5.24	Relation between indirect tensile strength and cube compressive strength for PAC	173
5.25	Relation between flexural tensile strength and cube compressive strength of moist cured PAC	173
5.26	Comparison of experimental strength with predicted strengths based on Bache's model for PAC-Addition with NWCA	177
5.27	Comparison of PAC experimental strengths with strengths of PAC without NWCA predicted from relative density and strength of reference mortar	177
5.28a	Typical stress-strain relation in compression for PACs with NWCA and corresponding reference concrete: mixes 20CA0, 20CA1, 20CA2, 20CA3, and 20CA4.	180
5.28b	Typical relation between stress/strength and strain for PACs with NWCA and corresponding reference concrete: mixes 20CA0, 20CA1, 20CA2, 20CA3, and 20CA4	180
5.29a	Typical stress-strain relation in compression for PACs without NWCA and corresponding reference mix: mixes 1MA0, 1MA1, 1MA2, 1MA3, and 1MA4	181

5.29b	Typical relation between stress/strength and strain for PACs with NWCA and corresponding reference concrete: mixes 1MA0, 1MA1, 1MA2, 1MA3, and 1MA4	181
5.30	Comparison of stress-strain relation in compression for PAC containing NWCA with that for NWC of similar compressive strength	182
5.31	Comparison of stress-strain relation for PACs containing NWCA with that for PAC without NWCA having similar compressive strength.	182
5.32	Variation of strain at peak stress with concrete strength	185
5.33	Variation of strain at peak stress with concrete density	185
5.34a	Comparison of stress-strain formulae (Eqns. 5.40 and 5.41) for ascending branch of stress-strain curve for PAC and NWC: composition of reference mix; cement content = 422 kg/m ³ , aggregate/cement ratio = 4.02, and w/c = 0.55)	187
5.34b	Comparison of stress-strain formulae (Eqns. 5.42 and 5.43) for ascending branch of stress-strain curve for PAC and NWC: composition of reference mix; cement content = 422 kg/m ³ , aggregate/cement ratio = 4.02, and w/c = 0.55)	188
5.35	Comparison of predicted boundary conditions of fitted stress/strength ratio-strain curves with experimental results	192
5.36	Variation of Static modulus with strength for PAC having different mix composition	195
5.37	Variation of static modulus of elasticity with cylinder compressive strength for PAC: regression curve fitted to all data regardless of specimen age	195
5.38	Variation of dynamic modulus of elasticity with cylinder compressive strength for PAC: regression curve fitted to all data regardless of specimen age	198
5.39	Linear regression relation between static modulus of elasticity and compressive strength: all ages	198
5.40	Variation of static modulus of elasticity with the density of concrete: reference concretes; 16CA0, 17CA0, 18CA0, 19CA0, and 20CA0	200
5.41	Regression of relative static chord modulus of elasticity on relative density of PAC at 28 days	201
5.42	Regression of relative dynamic modulus of elasticity on relative density of PAC at 28 days	201

5.43	Relation between 28-day predicted and measured moduli of elasticity for PAC	204
5.44	Relation between 28-day static and dynamic moduli of elasticity	206
5.45	Poisson's ratio as a function of compressive strength: mean value based on all data show	208
5.46	Poisson's ratio as a function of concrete density: mean value based on all data show	209
5.47	Poisson's ratio as a function of modulus of elasticity: mean value based on all data show	210
5.48	Influence of volumetric proportion of PAC on static chord modulus of PAC-Addition as estimated by various methods: cement content = 598 kg/m ³	216
5.49	Influence of volumetric proportion of PAC on static chord modulus of PAC-Addition as estimated by various methods: cement content = 422 kg/m ³	216
5.50	Influence of volumetric proportion of PAC on static chord modulus of PAC-Addition as estimated by various methods: cement content = 422 kg/m ³	217
5.51	Influence of volumetric proportion of PAC on static chord modulus of PAC (mortar reference mix) as estimated by various methods	217
6.1	Shrinkage of concretes of fixed paste content but with varying proportion of coarse normal weight aggregate and polystyrene aggregate: cement content = 450 kg/m ³ ; water/cement ratio = 0.40	224
6.2	Shrinkage of concretes of fixed paste content but with varying proportion of coarse normal weight aggregate and polystyrene aggregate: cement content = 545 kg/m ³ ; water/cement ratio = 0.40	224
6.3	Influence of concrete density on the rate of drying shrinkage strain development with time (expressed as the ratio of shrinkage at a given time to shrinkage at 520 days): w/c = 0.40	226
6.4	Influence of mix composition on shrinkage strain development (expressed as the average ratio of shrinkage at a given time to shrinkage at 520 days for PACs and reference concrete of the same mix proportions)	226

6.5	Relation between logarithm of shrinkage after 520 days of drying and polystyrene aggregate content: a/c = total aggregate/cement ratio; w/c = water/cement ratio; unit of shrinkage 1×10^{-6}	229
6.6	Influence of polystyrene aggregate content in concrete (by volume) on the ratio of PAC shrinkage to reference concrete shrinkage (relative shrinkage)	233
6.7	Influence of water/cement ratio and polystyrene aggregate content on PAC shrinkage: drying time = 520 days	233
6.8	Relation between drying shrinkage after 240 days and dynamic modulus of elasticity of concrete at 28 days	235
6.9	Relation between shrinkage and drying time for PAC moist cured for 7 and 30 days: PA content = 9.7% (by volume); nominal concrete density = 2000 kg/m ³	237
6.10	Relation between shrinkage and drying time for PAC moist cured for 7 and 30 days: PA content = 16.2% (by volume); nominal concrete density = 1800 kg/m ³	237
6.11	Relation between drying shrinkage and time for PAC stored at 50% and 75% relative humidity	240
6.12	Influence of mix composition on the ultimate drying shrinkage	244
6.13	Variation of ultimate drying shrinkage with density of concrete	244
6.14	Relation between relative ultimate shrinkage and relative concrete density	246
6.15	Variation of the coefficient, k (Eqn. 6.8) with drying time	248
6.16	Typical behaviour of PAC and NWC on drying and rewetting	249
6.17	Effect of concrete density on drying shrinkage and irreversible shrinkage: drying time = 520 days; rewetting time 177 days	249
6.18	Influence of concrete density on specific creep: aggregate/cement ratio = 2.89	257
6.19	Influence of concrete density on specific creep: aggregate/cement ratio = 3.94	257
6.20	Creep of PAC stored at 50% relative humidity: cement content 545 kg/m ³ ; age at application of load = 28 days; stress/strength ratio = 0.3	261
6.21	Creep of PAC stored at 50% relative humidity: cement content 425 kg/m ³ ; age at application of load = 28 days; stress/strength ratio = 0.3	261
6.22	Relation between logarithm of specific creep after 28, 56, and 150 days under load and polystyrene aggregate content	266

6.23	Influence of age at application of load on creep of PAC after 150 days under load	269
6.24	Comparison of the specific creep of PAC load at the ages of 28 days and 410 days	269
6.25	Influence of age at loading on the rate of creep for the first few days under load	271
6.26	Influence of curing and storage conditions on specific creep: aggregate/cement ratio = 2.89	272
6.27	Influence of curing and storage conditions on specific creep: aggregate/cement ratio = 3.94	273
6.28	Determination of constants of creep prediction expression: cement content = 545 kg/m ³ ; water content = 230 kg/m ³	277
6.29	Determination of constants of creep prediction expression: cement content = 425 kg/m ³ ; water content = 220 kg/m ³	277
6.30	Comparison of predicted creep strain to observed creep strain for various times under load	279
6.31	Development of creep coefficient with time under load: cement content = 545 kg/m ³ ; water content = 230 kg/m ³	283
6.32	Development of creep coefficient with time under load: cement content = 425 kg/m ³ ; water content = 220 kg/m ³	283
6.33	Variation of creep coefficient with concrete density (SSD)	284
6.34	Creep and creep recovery of PAC and reference concrete: compressive strength of reference concrete = 58.8 MPa	288
6.35	Creep and creep recovery of PAC and reference concrete: compressive strength of reference concrete = 43.0 MPa	288
6.36	Influence of density of concrete on creep and creep recovery	291
6.37	Influence of age of concrete at the time of loading on creep recovery of PAC	293
7.1	Relation between bond and compressive strengths: vertical bars	317
7.2	Relation between bond and compressive strengths: horizontal bars	317
7.3	Relation between bond and tensile strengths	318
7.4	Comparison Eqn. (7.1) and (7.1a): vertically embedded reinforcement	320
7.5	Comparison of Eqn. (7.2) and (7.2a): horizontally embedded reinforcement	320

7.6	Relation between relative bond strength and relative density for vertical and horizontal bars	323
7.7	Bond stress at various slips at loaded end of bar	328
7.8	Relation between slip at the loaded end and bond stress of vertically embedded bars: cement content = 425 kg/m ³ ; water content = 220 kg/m ³ .	329
7.9	Relation between slip at the loaded end and bond stress of vertically embedded bars: cement content = 545 kg/m ³ ; water content = 230 kg/m ³ .	329
7.10	Relation between development length and compressive strength based on pull-out test: vertical bars	340
7.11	Relation between development length and compressive strength based on pull-out test: horizontal bars	340
7.12	Ratio of test development length to calculated development length: vertical bars	341
7.13	Ratio of test development length to calculated development length: horizontal bars	341
8.1	Response of saturated cement paste to freezing and thawing both with and without entrained air	345
8.2	Hard and soft impact	355
8.3	Load-deformation response for an ideal energy absorbing material	355
8.4	Simple mechanical model for two-mass system	357
8.5	Single mass model	357
8.6	Variation of dynamic modulus of elasticity of PACs subjected to freezing and thawing as a function of cement content and testing procedure	364
8.7	Variation of ultrasonic pulse velocity of PACs subjected to freezing and thawing as a function of cement content and testing procedure	364
8.8	Variation of shrinkage of PACs subjected to freezing and thawing as a function of cement content and testing procedure	368
8.9	Variations density of PACs subjected to freezing and thawing as a function of cement content and testing procedure	368
8.10	Increase in dynamic modulus of elasticity of PAC subjected to freezing and thawing as a function of density and cement content	370

8.11	Increase in ultrasonic pulse velocity of PAC subjected to freezing and thawing as a function of density and cement content	370
8.12	Influence of density and cement content on relative dynamic modulus of elasticity of PAC subjected to freezing and thawing	371
8.13	Influence of density and cement content on relative pulse velocity of PAC subjected to freezing and thawing	371
8.14	Influence of density and cement content on variation of shrinkage of PAC subjected to freezing and thawing	372
8.15	Influence of concrete density and cement content on variation of density of PAC subjected to freezing and thawing	372
8.16	Comparison of variation of dynamic modulus of elasticity of PAC-Addition and PAC-Replacement (without normal weight coarse aggregate) subjected to freezing and thawing with reference mortar with and without air entrainment	375
8.17	Comparison of variation of pulse velocity of PAC-Addition and PAC-Replacement (without normal weight coarse aggregate) subjected to freezing and thawing with reference mortar with and without air entrainment	376
8.18	Comparison of freeze-thaw resistance of PAC-Addition (without normal weight coarse aggregate) with reference mortar with and without air entrainment	377
8.19	Comparison of freeze-thaw resistance of PAC-Replacement (without normal weight coarse aggregate) with reference mortar with and without air entrainment	377
8.20	Comparison of length change of PAC-Replacement (without normal weight coarse aggregate) subjected to freezing and thawing with reference mortar with and without air entrainment	379
8.21	Change in volume of frost-resistant and vulnerable concretes on cooling	379
8.22	Comparison of the spread of number of blows required for failure for 4.56 kg and 3.1 kg hammers	381
8.23	Effect of changes in cement content on impact strength of PAC	387
8.24	Relation between concrete density of plain PAC and number of blows to caused first crack and the depth of penetration after first crack	389
8.25	Effect of curing conditions on impact strength of plain PAC	391
8.26	Effect of curing conditions on impact strength of fibre reinforced PAC	391

8.27	Effect of prolonged curing on the impact resistance of plain PAC	394
8.28	Effect of prolonged curing on the impact resistance of fibre reinforced PAC	394
8.29	Effect of replacing sand of a reference mortar with PA on impact properties	397
8.30	Effect of the addition of PA to a mortar reference concrete on impact properties	397
8.31	Comparison of impact strength of plain PAC with fibre reinforced PAC: cement content = 425 kg/m ³ ; 28 days water cured	401
8.32	Comparison of impact strength of plain PAC with fibre reinforced PAC: cement content = 370 kg/m ³ ; 28 days water cured	401
8.33	Comparison of depth of penetration after first crack of plain PAC with fibre reinforced PAC	402
9.1	28-day cylinder compressive strength in relation to the unit weight of fresh polystyrene aggregate concrete for various reference concrete strengths.	409
9.2	Relation between relative cylinder strength and relative unit weight	409
9.3	Relation between the compressive strengths of polystyrene aggregate concrete containing standard and premium grades aggregates	411
9.4	Relation between cement content and 28-day compressive strength	414
9.5	Relation between water/cement ratio and 28-day compressive strength	414
9.6	Relationship between standard deviation and compressive strength	415
9.7	Relation between the compaction index and the water content	417
9.8	Void and air content in relation to relative unit weight of polystyrene aggregate concrete	420
9.9	Examples of the process for adjusting polystyrene aggregate content using the results of a trial mix	424
9.10	Illustration of the process of polystyrene aggregate content adjustment	426

9.11	Relation between 28-day cylinder compressive strength and unit weight of polystyrene aggregate concrete without normal weight coarse aggregate	429
9.12	Void and air contents per unit volume of concrete in relation to relative unit weight of polystyrene aggregate concrete without normal weight coarse aggregate	429

LIST OF TABLES

Table	Title	Page
2.1	Properties of polystyrene concrete	7
3.1	Composition of KANDOS brand, Type GP Portland Cement.	12
3.2	Mix proportion of PAC-Replacement with normal weight coarse aggregate	16
3.3	Mix proportion of PAC-Addition with normal weight coarse aggregate	19
3.4	Mix proportion of PAC without normal weight coarse aggregate	20
4.1	Summary of single-factor analysis of variance (ANOVA) of the effect of diameter of test container on compaction index	50
4.2	Summary of analysis of variance (ANOVA) for the effect of method of compaction on the compaction test	53
4.3	Mix proportions of polystyrene aggregate concrete of similar paste content (water/cement ratio = 0.40)	61
4.4	Apparent workability ranges for polystyrene aggregate concrete based on the compaction test, air bubble escape time and visual judgement.	76
4.5	Description of workability and magnitude of slump of PAC compared with that of normal weight concrete	82
4.6	Repeatability of the DIN compaction test	84
4.7	Descriptive statistics of repeatability test results	84
4.8	Unit weights for concretes having different cement contents	93
4.9	Difference between calculate and measured unit weights	93
4.10	Sums of Squares and Cross-Products for concretes having different cement contents	95
4.11	Analysis of Covariance for the effect of cement content	95
4.12	Analysis of covariance for the effect of water/cement ratio on the unit weight of polystyrene aggregate concrete	101
4.13	Comparison of lines of regression for concretes with polystyrene aggregate replacement and addition	104
4.14	Summary of analysis of covariance for the comparison of lines of regression for polystyrene aggregate concretes with concrete and mortar reference mixes	107

4.15	Regression of measured unit weight on calculated unit weight and analysis of variance for PAC with normal weight coarse aggregate	110
4.16	Regression of measured unit weight on calculated unit weight and analysis of variance for PAC without normal weight coarse aggregate	110
4.17	Error associated with using simplified equations of regression for measured unit weight on calculated unit weight	115
5.1	Properties of hardened PAC-Addition at the age of 28 days	131
5.2	Properties of hardened PAC-Replacement at the age of 28 days	132
5.3	28-day Properties of hardened PAC not containing coarse normal weight aggregate (mortar reference mix)	133
5.4	28-day compressive strength and density of PAC-Addition compared to corresponding reference concrete	134
5.5	28-day compressive strength and density of PAC-Replacement compared to corresponding reference concrete	135
5.6	28-day compressive strength and density of PAC (mortar reference mixes) compared to corresponding reference concrete	135
5.7	Regression equation connecting relative strength and relative density	141
5.8	Relative gain of compressive strength (PAC-Addition with NWCA)	150
5.9	Relative gain of strength (PAC-Replacement with NWCA)	151
5.10	Relative gain of strength (PAC-Replacement without NWCA)	151
5.11	Regression equation relating compressive strength (MPa) and age (days) for PAC with NWCA	154
5.12	Regression equation connecting compressive strength (MPa) and age (days) for PAC without NWCA	154
5.13	Indirect tensile strength of PAC-Addition compared with corresponding reference concrete at the age of 28 days	162
5.14	Indirect tensile strength of PAC-Replacement compared with corresponding reference concrete at the age of 28 days	163
5.15	Regression equation connecting relative indirect tensile strength and relative density of PAC	167
5.16	28-day relative compressive strength compared with 28-day relative indirect tensile strength	167

5.17	Empirical constants for expressions relating indirect tensile strength to compressive strength	169
5.18	Comparison of experimental and theoretical 28-day compressive strength of PAC-Addition	175
5.19	Comparison of experimental and theoretical compressive strength of PAC-Replacement	176
5.20	Modulus of elasticity of concretes of similar compressive strength	184
5.21	Comparison of formulae for stress-strain curve of concrete	190
5.22	Comparison of predicted and experimental strain and modulus of elasticity	191
5.23	Influence of aggregate content on static and dynamic moduli of PAC of comparable compressive strength	194
5.24	Regression expression relation modulus and cylinder compressive strength	197
5.25	Regression equations relating the experimental static and dynamic modulus of elasticity and relative density	202
5.26	Regression equations relating modulus to density and compressive strength of concrete	202
5.27	Poisson's ratio of PAC and reference NWC	212
5.28	Comparison of observed and predicted modulus of elasticity of PAC	214
5.29	Ratios of estimated modulus to observed modulus	215
6.1	Development of shrinkage of PAC with drying time	225
6.2	Comparison of observed shrinkage and calculated shrinkage from Eqn. 6.4	228
6.3	Regression equation relating shrinkage of PACs to shrinkage of reference concrete, for various mix composition and ages	231
6.4	Influence of water/cement ratio and aggregate/cement ratio on drying shrinkage	232
6.5	Effect of concrete properties on drying shrinkage of PAC	236
6.6	Influence of initial water curing duration on shrinkage of PAC	238
6.7	Effect of drying condition on shrinkage	240
6.8	Empirical constants for drying shrinkage prediction	243
6.9	Effect of polystyrene aggregate content and water/cement ratio on ultimate shrinkage	245
6.10	Effect of density on ultimate shrinkage	245

6.11	Variation of exponential coefficient, k (Eqn. 6.8) with time	248
6.12	Reversible and irreversible part of shrinkage of PAC and NWC	250
6.13	Comparison of shrinkage of PAC with other lightweight and normal weight concretes	252
6.14	Instantaneous strain on loading	255
6.15	Summary of creep properties at 150 days under load	258
6.16	Effect of cement paste content on creep strain	262
6.17	Influence of strength of concrete on creep of PAC	263
6.18	Values of α and specific creep for various cement paste content and ages	267
6.19	Effect of curing and storage conditions	275
6.20	Constants for predicting the creep of PAC loaded at the age of 28 days	278
6.21	Predicted creep properties	281
6.22	Strength and modulus of elasticity of lightweight aggregates concrete	286
6.23	Creep recovery of concrete loaded at stress/strength of 0.3 and maintained for 150 days	290
6.24	Comparison of predicted ultimate creep coefficient	297
6.25	Comparison of predicted ultimate shrinkage strain	297
7.1	Bond strength of PAC compared with reference normal weight concrete	315
7.2	Comparison of Eqn. (7.2) and (7.2a) relating compressive strength and bond strength for horizontally embedded reinforcement	321
7.3	Effect of concrete density on bond strength of vertical bars	325
7.4	Effect of concrete density on bond strength of horizontal bars	327
7.5	Effect of orientation of bar on bond strength	332
7.6	Influence of bar position on bond strength	333
7.7	Development length for vertically embedded bars	337
7.8	Development length for horizontally embedded bars	338
7.9	Development length as a function of bar position	339
8.1	Properties of fresh concretes used in freezing and thawing investigation	362
8.2	Results of freezing and thawing tests (Series 1)	363

8.3	Relative dynamic modulus of elasticity and pulse velocity (Series 1)	366
8.4	Summary of freezing and thawing test results for PAC without normal weight coarse aggregate (Series 2)	374
8.5	Relative dynamic modulus of elasticity (Series 2)	376
8.6	Impact strength of 28 days water cured PAC	384
8.7	Effect of the moisture conditions of concrete on impact strength	385
8.8	Effect of curing duration on impact strength	386
8.9	Effect of storage conditions on impact resistance	392
8.10	Effect of duration of curing on impact strength	396
8.11	Effect of method of polystyrene aggregate incorporation on impact response	398
8.12	Effect of fibre reinforcement on impact strength of PAC after 28 days of water curing	400
9.1	Properties of materials	430

LIST OF PLATES

Plate	Title	Page
4.1	Appearances of polystyrene aggregate concrete having various workability	72
7.1	Typical failure modes for reference concretes	310
7.2	Typical failure modes for PAC having nominal density of 2000 kg/m ³	310
7.3	Typical failure modes for PAC having nominal density of 1800 kg/m ³	311
7.4	Typical failure modes for PAC having nominal density of 1600 kg/m ³	311
7.5	Typical deformation of concrete around reinforcing bar for reference concretes	312
7.6	Typical deformation of concrete around reinforcing bar for PAC having nominal density of 2000 kg/m ³	312
7.7	Typical deformation of concrete around reinforcing bar for PAC having nominal density of 1800 kg/m ³	313
7.8	Typical deformation of concrete around reinforcing bar for PAC having nominal density of 1600 kg/m ³	313
8.1	Typical modes of failure of plain PAC with normal weight coarse aggregate (left side) and PAC without normal weight coarse aggregate (right side) under repeated impact.	382

NOTATION

c	=	Creep of concrete
c_p	=	Creep of paste
E_a	=	Modulus of elasticity of aggregate
E_c	=	Static modulus of elasticity
E_d	=	Dynamic modulus of elasticity
E_m	=	Modulus of elasticity of matrix (or mortar)
$f_{c,28}$	=	Compressive strength of concrete at 28 days
f_{cyl}	=	Cylinder compressive strength
f_{cf}	=	Flexural tensile strength of concrete
f_{ct}	=	Indirect tensile strength
f_{cu}	=	Cube compressive strength
f_o	=	Compressive strength of reference concrete
G_d	=	Dynamic modulus of rigidity
g	=	Volumetric content of aggregate
K	=	Bulk modulus of elasticity
k	=	A coefficient, ratio or factor used with and without numerical subscripts
p	=	Porosity
t	=	Time
α	=	A coefficient
β	=	A coefficient with or without numerical subscripts
Δ	=	Positive or negative increment
ϵ	=	Strain
ϵ_i	=	Instantaneous strain
ϵ_c	=	Strain due to concrete creep
ϵ_{sh}	=	Strain due to shrinkage
$\epsilon_{sh,o}$	=	Shrinkage strain of reference concrete
ϵ_{sp}	=	Total load induced strain per MPa
$\phi(t)$	=	Creep coefficient
μ	=	Poisson's ratio
λ	=	Wavelength of vibration
ρ	=	Density of concrete
ρ_o	=	Density of reference concrete

ABBREVIATIONS

ACI	=	American Concrete Institute
ASTM	=	American Society for Testing and Materials
CEB	=	Euro-International Committee for Concrete
CUR	=	Commissie voor Uitvoering van Research
CUW	=	Calculated unit weight
DOE	=	Department of the Environment (Building Research Establishment, Watford, UK)
FIP	=	International Federation for Prestressing
LWC	=	Lightweight concrete
LWAC	=	Lightweight aggregate concrete
MUW	=	Measured unit weight
NWC	=	Normal weight concrete
PA	=	Polystyrene aggregate
PAC	=	Polystyrene aggregate concrete
RILEM	=	International Union of Testing and research laboratories for Materials and Construction